

STRUCTURAL WIND CALCULATIONS

For

CAFÉ HOLLANDER Milwaukee, WI

For

The Kubala Washatko Architects



By



Alan T. Rentmeester, PE

SPIRE ENGINEERING, INC.
305 N Plankinton Ave. Suite 101
Milwaukee, WI 53203
(414) 278-9200

April 10, 2024

Wind Calculation Summary

WIND LOAD INFORMATION:

BASIC WIND SPEED		115 MPH	
RISK CATEGORY		II	
WIND EXPOSURE		B	
INTERNAL PRESSURE COEFFICIENTS		± 0.18	
COMPONENTS AND CLADDING (STRENGTH DESIGN/ULTIMATE LOADS)			
WIDTH OF PRESSURE COEFFICIENT ZONE (a)		5.0 ft	
TRIBUTARY WIND LOAD AREAS:	<u>10 ft²</u>	<u>50 ft²</u>	<u>100 ft²</u>
WALLS:			
ZONE 4	-23.6 psf	-21.3 psf	-20.4 psf
ZONE 5	-29.0 psf	-24.6 psf	-22.6 psf
POSITIVE PRESSURE ALL ZONES	21.8 psf	19.5 psf	18.6 psf

Code Search

Code: Wisconsin Building Code 2018

Occupancy:

Occupancy Group = B Business

Risk Category & Importance Factors:

Risk Category = II
Wind factor = 1.00
Snow factor = 1.00
Seismic factor = 1.00

Type of Construction:

Fire Rating:
Roof = 0.0 hr
Floor = 0.0 hr

Building Geometry:

Roof angle (θ) 0.00 / 12 0.0 deg
Building length (L) 55.0 ft
Least width (B) 50.0 ft
Mean Roof Ht (h) 30.0 ft
Parapet ht above grd 30.0 ft
Minimum parapet ht 0.0 ft

Live Loads:

Roof
0 to 200 sf: 20 psf
200 to 600 sf: 24 - 0.02Area, but not less than 12 psf
over 600 sf: 12 psf

Awnings and canopys - non fabric 20 psf

Floor:

Typical Floor 40 psf
Partitions 15 psf
Stairs and exit ways 100 psf
Public rooms & corridors serving ther 100 psf

Spire Engineering, Inc.

305 N. Plankinton, Suite 101
Milwaukee, WI 53203
(414) 278 - 9200

JOB TITLE Café Hollander Wind Loads

JOB NO. _____ SHEET NO. _____
CALCULATED BY Spire Engineering DATE _____
CHECKED BY _____ DATE _____

Wind Loads :

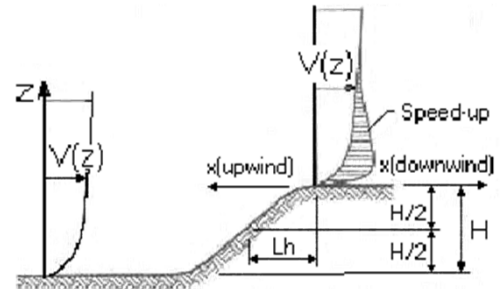
ASCE 7 - 10

Ultimate Wind Speed	115 mph
Nominal Wind Speed	89.1 mph
Risk Category	II
Exposure Category	B
Enclosure Classif.	Enclosed Building
Internal pressure	+/-0.18
Directionality (Kd)	0.85
Kh case 1	0.701
Kh case 2	0.701
Type of roof	Monoslope

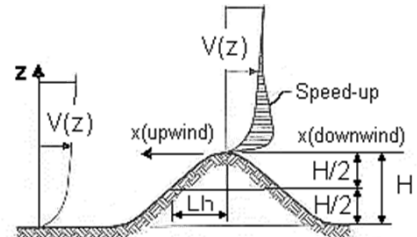
Topographic Factor (Kzt)

Topography	Flat
Hill Height (H)	0.0 ft
Half Hill Length (Lh)	0.0 ft
Actual H/Lh =	0.00
Use H/Lh =	0.00
Modified Lh =	0.0 ft
From top of crest: x =	50.0 ft
Bldg up/down wind?	downwind
H/Lh = 0.00	K ₁ = 0.000
x/Lh = 0.00	K ₂ = 0.000
z/Lh = 0.00	K ₃ = 1.000
At Mean Roof Ht:	K _z t = (1+K ₁ K ₂ K ₃) ² = 1.00

H < 60ft; exp B
∴ K_zt = 1.0



ESCARPMENT



2D RIDGE or 3D AXISYMMETRICAL HILL

Gust Effect Factor

h =	30.0 ft
B =	50.0 ft
/z (0.6h) =	30.0 ft

Flexible structure if natural frequency < 1 Hz (T > 1 second).
If building h/B > 4 then may be flexible and should be investigated.
h/B = 0.60 Rigid structure (low rise bldg)

G = 0.85 Using rigid structure default

Rigid Structure

\bar{e} =	0.33
l =	320 ft
Z _{min} =	30 ft
c =	0.30
g _Q , g _v =	3.4
L _z =	310.0 ft
Q =	0.89
I _z =	0.30
G =	0.86 use G = 0.85

Flexible or Dynamically Sensitive Structure

3rd cy (η ₁) =	0.0 Hz
Damping ratio (β) =	0
/b =	0.45
/α =	0.25
V _z =	74.1
N ₁ =	0.00
R _n =	0.000
R _n =	28.282
R _B =	28.282
R _L =	28.282
g _R =	0.000
R =	0.000
G _f =	0.000
η =	0.000
η =	0.000
η =	0.000
h =	30.0 ft

Enclosure Classification

Test for Enclosed Building:

A building that does not qualify as open or partially enclosed.

Spire Engineering, Inc.

305 N. Plankinton, Suite 101
 Milwaukee, WI 53203
 (414) 278 - 9200

JOB TITLE Café Hollandor Wind Loads

JOB NO. _____

SHEET NO. _____

CALCULATED BY Spire Engineering

DATE _____

CHECKED BY _____

DATE _____

Test for Open Building:

All walls are at least 80% open.
 $A_o \geq 0.8A_g$

Test for Partially Enclosed Building:

Predominately open on one side only

Input			Test	
Ao	500.0 sf		$A_o \geq 1.1A_{oi}$	NO
Ag	600.0 sf		$A_o > 4'$ or $0.01A_g$	YES
Aoi	1000.0 sf		$A_{oi} / A_{gi} \leq 0.20$	YES
Agi	10000.0 sf			

Building is NOT Partially Enclosed

Conditions to qualify as Partially Enclosed Building. Must satisfy all of the following:

- $A_o \geq 1.1A_{oi}$
- $A_o >$ smaller of 4' or 0.01 Ag
- $A_{oi} / A_{gi} \leq 0.20$

Where:

- Ao = the total area of openings in a wall that receives positive external pressure.
- Ag = the gross area of that wall in which Ao is identified.
- Aoi = the sum of the areas of openings in the building envelope (walls and roof) not including Ao.
- Agi = the sum of the gross surface areas of the building envelope (walls and roof) not including Ag.

Reduction Factor for large volume partially enclosed buildings (Ri) :

If the partially enclosed building contains a single room that is unpartitioned, the internal pressure coefficient may be multiplied by the reduction factor Ri.

Total area of all wall & roof openings (Aog): 0 sf
 Unpartitioned internal volume (Vi) : 0 cf
 Ri = 1.00

Altitude adjustment to constant 0.00256 (caution - see code) :

Grd level above sea level = 0.0 ft
 Constant = 0.00256
 Adj Constant = 0.00256
 Average Air Density = 0.0765 lbf/ft3

Spire Engineering, Inc.

305 N. Plankinton, Suite 101
Milwaukee, WI 53203
(414) 278 - 9200

JOB TITLE Café Hollander Wind Loads

JOB NO. _____
CALCULATED BY Spire Engineering
CHECKED BY _____

SHEET NO. _____
DATE _____
DATE _____

Ultimate Wind Pressures

Wind Loads - Components & Cladding : h ≤ 60'

Kh (case 1) = 0.70 h = 30.0 ft
Base pressure (qh) = **20.2 psf** a = 5.0 ft
Minimum parapet ht = 5.0 ft GCpi = +/-0.18
Roof Angle (θ) = 0.0 deg
Type of roof = Monoslope

Roof

Area	GCp +/- GCpi				Surface Pressure (psf)			
	10 sf	50 sf	100 sf	500 sf	10 sf	50 sf	100 sf	500 sf
Negative Zone 1	-1.18	-1.11	-1.08	-1.08	-23.8	-22.4	-21.8	-21.8
Negative Zone 2	-1.98	-1.49	-1.28	-1.28	-39.9	-30.0	-25.8	-25.8
Negative Zone 3	-1.98	-1.49	-1.28	-1.28	-39.9	-30.0	-25.8	-25.8
Positive Zone 1	0.48	0.41	0.38	0.38	16.0	16.0	16.0	16.0
Positive Zones 2 & 3	1.08	0.97	0.92	0.81	21.8	19.6	18.5	16.3
Overhang Zone 1&2	-1.7	-1.63	-1.6	-1.1	-34.3	-32.9	-32.3	-22.2
Overhang Zone 3	-1.7	-1.63	-1.6	-1.1	-34.3	-32.9	-32.3	-22.2

User input	
20 sf	50 sf
-23.2	-22.4
-35.7	-30.1
-35.7	-30.1
16.0	16.0
20.8	19.5
-33.7	-32.9
-33.7	-32.9

Negative zone 3 = zone 2, since parapet >= 3ft.

Overhang pressures in the table above assume an internal pressure coefficient (GCpi) of 0.0
Overhang soffit pressure equals adj wall pressure (which includes internal pressure of 3.6 psf)

Parapet

qp = 21.1 psf

Solid Parapet Pressure	Surface Pressure (psf)					
	10 sf	20 sf	50 sf	100 sf	200 sf	500 sf
CASE A: Zone 2 :	56.9	51.4	44.2	38.8	37.8	36.4
Zone 3 :	56.9	51.4	44.2	38.8	37.8	36.4
CASE B: Edge zones 2 :	-39.8	-37.8	-35.1	-33.1	-31.1	-28.4
Corner zones 3 :	-45.5	-42.5	-38.5	-35.5	-32.4	-28.4

User input
50 sf
44.2
44.2
-35.1
-38.5

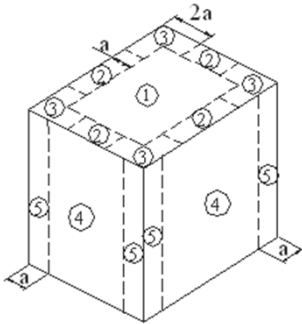
Walls

Area	GCp +/- GCpi				Surface Pressure (psf)			
	10 sf	100 sf	200 sf	500 sf	10 sf	100 sf	200 sf	500 sf
Negative Zone 4	-1.17	-1.01	-0.96	-0.90	-23.6	-20.4	-19.4	-18.1
Negative Zone 5	-1.44	-1.12	-1.03	-0.90	-29.0	-22.6	-20.7	-18.1
Positive Zone 4 & 5	1.08	0.92	0.87	0.81	21.8	18.6	17.6	16.3

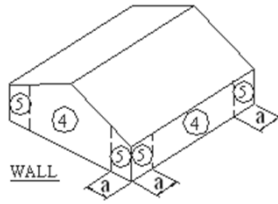
User input	
50 sf	150 sf
-21.3	-19.8
-24.6	-21.5
19.5	18.0

Note: GCp reduced by 10% due to roof angle <= 10 deg.

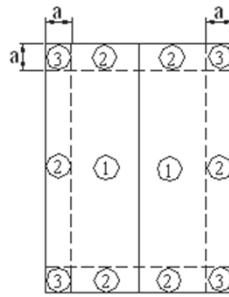
Location of C&C Wind Pressure Zones - ASCE 7-10 & earlier



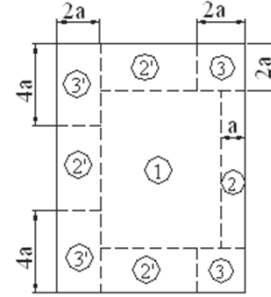
Roofs w/ $\theta \leq 10^\circ$
 and all walls
 $h > 60'$



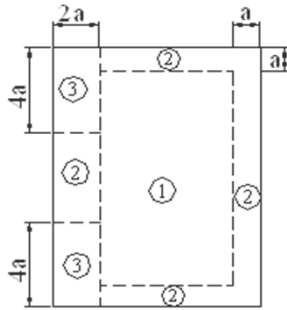
Walls $h \leq 60'$
 & alt design $h < 90'$



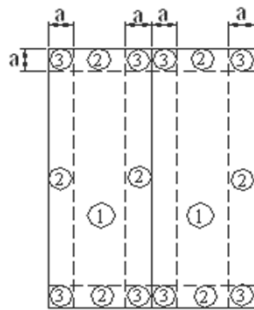
Gable, Sawtooth and
 Multispan Gable $\theta \leq 7$ degrees &
 Monoslope ≤ 3 degrees
 $h \leq 60'$ & alt design $h < 90'$



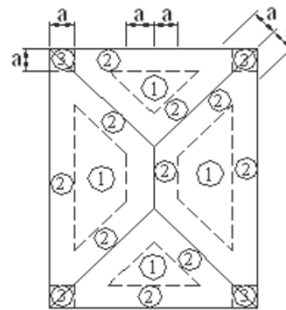
Monoslope roofs
 $3^\circ < \theta \leq 10^\circ$
 $h \leq 60'$ & alt design $h < 90'$



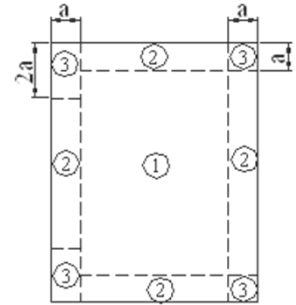
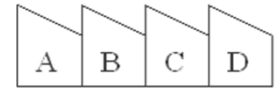
Monoslope roofs
 $10^\circ < \theta \leq 30^\circ$
 $h \leq 60'$ & alt design $h < 90'$



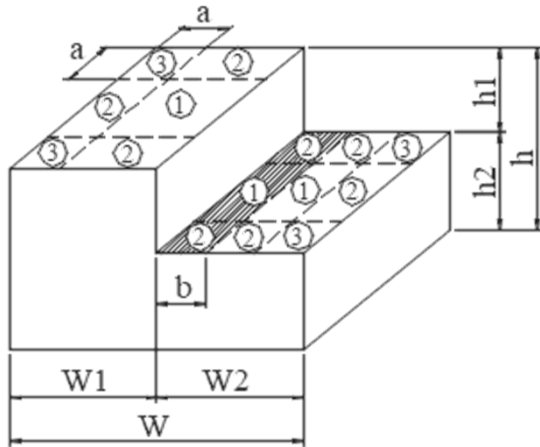
Multispan Gable &
 Gable $7^\circ < \theta \leq 45^\circ$



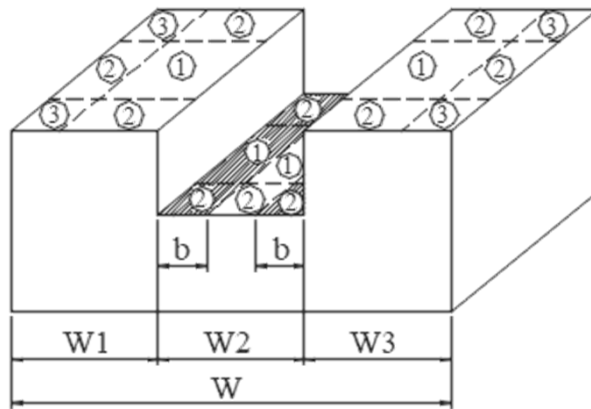
Hip $7^\circ < \theta \leq 27^\circ$



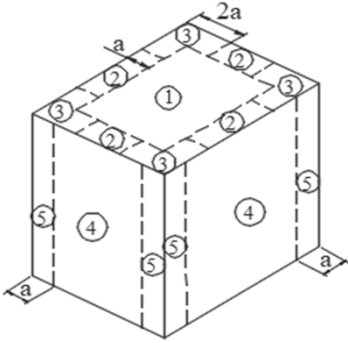
Sawtooth $10^\circ < \theta \leq 45^\circ$
 $h \leq 60'$ & alt design $h < 90'$



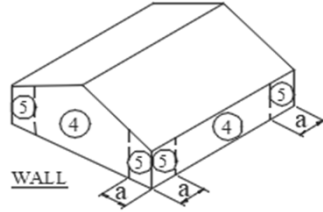
Stepped roofs $\theta \leq 3^\circ$
 $h \leq 60'$ & alt design $h < 90'$



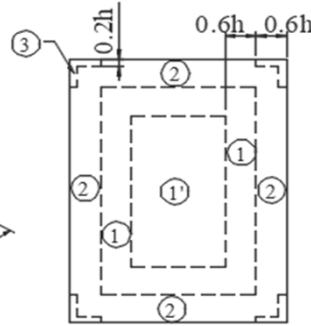
Location of C&C Wind Pressure Zones - ASCE 7-16



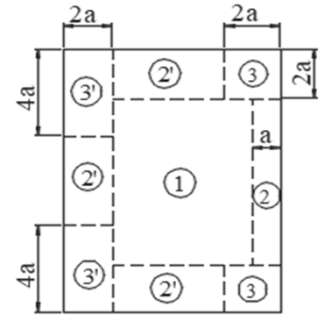
Roofs w/ $\theta \leq 10^\circ$
 and all walls
 $h > 60'$



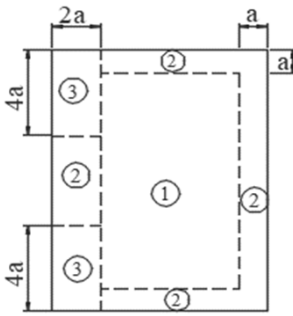
Walls $h \leq 60'$
 & alt design $h < 90'$



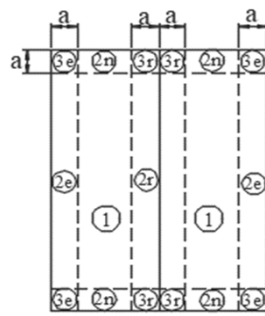
Gable, Sawtooth and
 Multispan Gable $\theta \leq 7$ degrees &
 Monoslope ≤ 3 degrees
 $h \leq 60'$ & alt design $h < 90'$



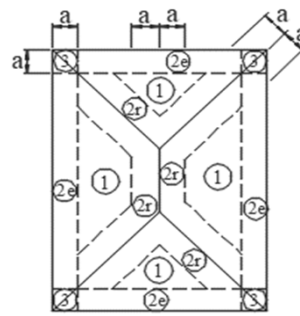
Monoslope roofs
 $3^\circ < \theta \leq 10^\circ$
 $h \leq 60'$ & alt design $h < 90'$



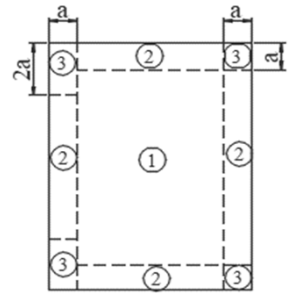
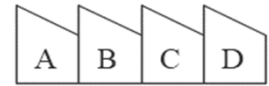
Monoslope roofs
 $10^\circ < \theta \leq 30^\circ$
 $h \leq 60'$ & alt design $h < 90'$



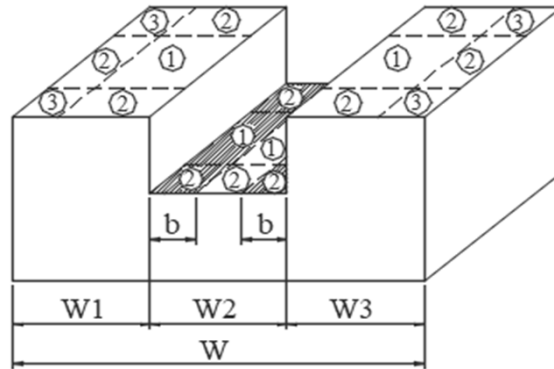
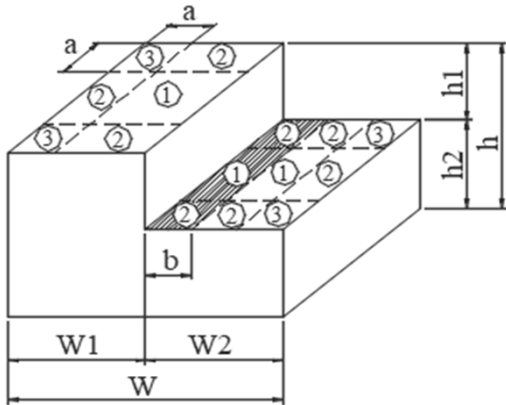
Multispan Gable &
 Gable $7^\circ < \theta \leq 45^\circ$



Hip $7^\circ < \theta \leq 27^\circ$



Sawtooth $10^\circ < \theta \leq 45^\circ$
 $h \leq 60'$ & alt design $h < 90'$



Stepped roofs $\theta \leq 3^\circ$
 $h \leq 60'$ & alt design $h < 90'$

⚠ This is a beta release of the new ATC Hazards by Location website. Please [contact us](#) with feedback.

ℹ The ATC Hazards by Location website will not be updated to support ASCE 7-22. [Find out why.](#)

ATC Hazards by Location

Search Information

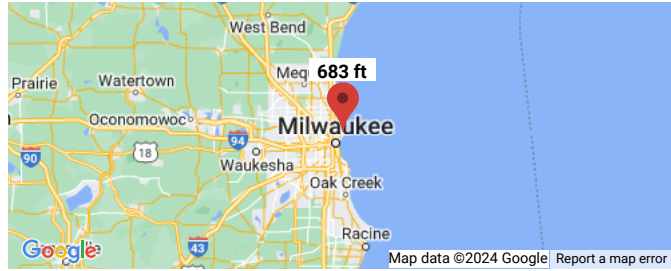
Address: 2608 N Downer Ave, Milwaukee, WI 53211

Coordinates: 43.0665232, -87.8778741

Elevation: 683 ft

Timestamp: 2024-04-10T14:39:54.551Z

Hazard Type: Wind



ASCE 7-16

MRI 10-Year 73 mph

MRI 25-Year 80 mph

MRI 50-Year 85 mph

MRI 100-Year 91 mph

Risk Category I 100 mph

Risk Category II 106 mph

Risk Category III 114 mph

Risk Category IV 118 mph

ASCE 7-10

MRI 10-Year 76 mph

MRI 25-Year 84 mph

MRI 50-Year 90 mph

MRI 100-Year 96 mph

Risk Category I 105 mph

Risk Category II 115 mph

Risk Category III-IV 120 mph

ASCE 7-05

ASCE 7-05 Wind Speed 90 mph

The results indicated here DO NOT reflect any state or local amendments to the values or any delineation lines made during the building code adoption process. Users should confirm any output obtained from this tool with the local Authority Having Jurisdiction before proceeding with design.

Please note that the ATC Hazards by Location website will not be updated to support ASCE 7-22. [Find out why.](#)

Disclaimer

Hazard loads are interpolated from data provided in ASCE 7 and rounded up to the nearest whole integer. Per ASCE 7, islands and coastal areas outside the last contour should use the last wind speed contour of the coastal area – in some cases, this website will extrapolate past the last wind speed contour and therefore, provide a wind speed that is slightly higher. NOTE: For queries near wind-borne debris region boundaries, the resulting determination is sensitive to rounding which may affect whether or not it is considered to be within a wind-borne debris region.

Mountainous terrain, gorges, ocean promontories, and special wind regions shall be examined for unusual wind conditions.

While the information presented on this website is believed to be correct, ATC and its sponsors and contributors assume no responsibility or liability for its accuracy. The material presented in the report should not be used or relied upon for any specific application without competent examination and verification of its accuracy, suitability and applicability by engineers or other licensed professionals. ATC does not intend that the use of this information replace the sound judgment of such competent professionals, having experience and knowledge in the field of practice, nor to substitute for the standard of care required of such professionals in interpreting and applying the results of the report provided by this website. Users of the information from this website assume all liability arising from such use. Use of the output of this website does not imply approval by the governing building code bodies responsible for building code approval and interpretation for the building site described by latitude/longitude location in the report.