



850 Forest Edge Drive, Vernon Hills, IL 60061

TEL 847.478.9700 ■ FAX 847.478.9701

820 Lakeside Drive, Suite 5, Gurnee, IL 60031
TEL 847.855.1110 FAX 847.855.1115

www.gha-engineers.com

To:

John Culligan

Cuningham Group Architecture, P.A.

From:

Bill Grieve

Date:

March 26, 2012

Subject:

Potawatomi Bingo Casino Hotel

Milwaukee, Wisconsin

PART I. PROJECT CONTEXT AND SUMMARY STATEMENT

GEWALT HAMILTON ASSOCIATES, INC. (GHA) has been working with the Potawatomi at their Bingo Casino in Milwaukee, Wisconsin since 1999. The next phase of expansion is construction of a 20-story luxury hotel with up to 385 rooms on the former Badger Railing site in the southwest corner of the Canal Street / Potawatomi Circle intersection (lower 16th Street). A four level, 480 parking deck will also be built directly east of the existing parking garages.

GHA has conducted a traffic impact analysis (TIA) for the Potawatomi Bingo Casino Hotel. The following summarizes our findings and provides various recommendations for your consideration. *Exhibits* and Appendices referenced are located at the end of this document. Briefly summarizing, we believe that the adjacent roadways and the site access drives can accommodate hotel traffic activity. Reasons include...

- The Potawatomi Bingo Casino is well served by the adjacent streets, which offer excellent route selection and site access flexibility.
- Potawatomi Circle functions as a loop traffic distributor for the Bingo Casino, directing traffic to/from the building entrances and parking garages and lots. This helps to minimize the traffic impact at any one location along Canal Street.
- It is expected that 50% of hotel trip will be made by gaming patrons of the Bingo Casino. However, this discount was not applied to ensure that the maximum hotel traffic impacts were tested.

PART II. BACKGROUND INFORMATION

Exhibits 1 and 2 - Site Location Map and Photo Inventory

Exhibit 1 provides aerial photos of the site vicinity and Exhibit 2 provides a photo inventory of current traffic operations. Patrons of the hotel will be well served by the adjacent streets to the Bingo Casino, which includes...

- Canal Street serves the entire Menomonee Valley, linking 6th Street in Downtown Milwaukee on the east with Miller Park on the west.
- The traffic signal at the Canal Street / Potawatomi Circle (formerly 20th Street) intersection on the west side of the Casino.
- Potawatomi Circle (formerly Pittsburgh Avenue) on the south side of the Bingo Casino that also provides direct access to the parking garages and lots.
- The traffic signal at the Canal Street / Potawatomi Circle (formerly lower 16th Street) intersection on the east side of the Casino.
- The traffic signal that provides direct access to the parking decks from upper 16th Street.

Exhibit 3 - Existing Traffic

GHA conducted traffic counts on Thursday, March 1, 2012 at the Bingo Casino access intersections from 2 PM to 9 PM. The length of the counts helped us to understand the shifts in Bingo Casino traffic activity and travel patterns. *Exhibit 3* illustrates the existing weekday evening peak hour traffic volumes, which occurred from 5 PM to 6 PM.

Pertinent comments from the counts and our observations include...

- About 50% of the Bingo Casino trips are oriented to/from the west on Canal Street. This
 can be partially attributable to the convenience and close proximity of the 25th Street
 interchange with I-94.
- Traffic in and out of the Bingo Casino ranged from a low of 1525 vehicles from 2 PM to 3 PM to a high of 1825 vehicles during the 5 PM to 6 PM peak hour and 1800 vehicles from 6 PM to 7 PM. The traffic activity then gradually decreased with 1610 trips generated from 8 PM to 9 PM.

PART III. TRAFFIC CHARACTERISTICS

Exhibit 4 - Site Plan and Proposed Access

Exhibit 4 illustrates the proposed hotel site plan. Planned access includes...

- Hotel traffic will be able to use all of the patron garages.
- The drives on Canal Street and Potawatomi Circle that will be shifted slightly from their current location, and will provide valet service for the main hotel entrance.
- The existing valet service for the Bingo Casino will be shifted to the east-west portion of Potawatomi Circle, so as not to mix drop-off and pick-up functions.

Exhibit 5 - Project Traffic Characteristics

Traffic Generations

Traffic was generated for the hotel, based on rate information published by the Institute of Transportation Engineers (ITE). Exhibit 5 – Part A summarizes the calculated weekday morning and evening and daily traffic generations.

As can be seen from Exhibit 5 - Part A, and per information provided by the owner, it can be expected that about 50% of hotel guests will already be on-site gaming and enjoying the many entertainment opportunities offered at the Potawatomi Bingo Casino. However, this discount was not taken to ensure that the maximum potential hotel traffic impacts are tested.

Trip Distribution

As noted, the hotel will be served well by the several means of accessing the Potawatomi Bingo Casino. This will help minimize the traffic impacts by distributing the hotel trips over several locations. Exhibit 5 – Part B summarizes the trip distribution, which is based on the traffic volumes and travel patterns illustrated on Exhibit 3.4 – Existing Traffic 5 PM to 6 PM.

PART IV. TRAFFIC EVALUATION

Exhibits 6 and 7 - Site and Total Traffic Assignments

Exhibit 6 illustrates the site traffic assignment, which is based on the project traffic characteristics summarized in Exhibit 5 (e.g. traffic generations and trip distribution) and the site access drives. Site traffic and background volumes (see Exhibits 3 and 6) were combined to produce the Total Traffic Assignment, which is illustrated in Exhibit 7.

Pertinent comments on the traffic assignments include...

- About 65% of hotel traffic is anticipated to use the convenience of the valet service and/or to drop-off and pick-up passengers and luggage at the hotel porte cochere prior to parking in one of the garages.
- The remaining 35% of hotel guests would use the direct access to the garage on upper 16th Street. These trips would probably be made by hotel patrons who don't have much luggage and by Bingo Casino gamers who decide to check-in at a later time.
- The total traffic assignment (see *Exhibit 7*) reflects the change in the current Bingo Casino valet drop-off / pick-up operations. This will add westbound left turns and eastbound right turns on Canal Street at Potawatomi Circle (lower 16th Street) that now use the existing valet.

Exhibit 8 - Intersection Capacity Analyses

Intersection capacity analyses were conducted, per guidelines in the Highway Capacity Manual (HCM). At signalized intersections, Level of Service (LOS) reports operations using the letter designations "A" (best) through "F" (worst) and measures the average control delay per vehicle in seconds. At unsignalized intersections where the minor approaches have stop control, the HCM measurement is approach delay in seconds, with the results reported from LOS A to LOS F.

Exhibit 8 summarizes the intersection capacity analysis results for the weekday evening peak hour. The software printouts are also provided in the *Appendix* for reference. Pertinent comments include...

- All intersections tested will continue to operate better than the design LOS C. In fact, site
 traffic has less than a one second impact on the overall delay at all intersections during
 the weekday evening peak hour.
- The 95th percentile queue for eastbound Canal at Potawatomi Circle (lower 16th Street) is 102 feet for existing traffic and 122 feet for the total traffic.

<u>Key Findings.</u> Based on the results of the capacity analyses, no road improvements are needed to specifically accommodate hotel traffic. And the eastbound queue on Canal Street will not extend past the new valet entrance. Thus, our recommendations focus on the access operations and site plan elements.

PART V. SITE PLAN ELEMENTS

Hotel Access Recommendations

Canal Street Valet Entrance

- 1. The drive will be relocated a bit to the east from its current location, about 150 feet west of Potawatomi Circle. Access will be limited to inbound movements only with a one-way counterclockwise circulation pattern.
- Two inbound lanes will be provided from Canal, then will widen in the porte cochere influence area for four lanes, including valet lanes, a through travel lane, and a bus staging area for luggage and passenger unloading.
- 3. The median on Canal Street should be modified to close the existing opening. A new opening and westbound left turn lane should be provided. The site plan shows that about three cars of stacking can be provided, which should be adequate to accommodate the anticipated volumes (see *Exhibits 6 and 7*).
- 4. The modified island should be designed so as to physically preclude a driver from attempting to exit via the entrance.

Potawatomi Circle (lower 16th Street) Valet Exit

- 5. The valet exit will also be relocated a bit to the north onto Potawatomi Circle. Two exit lanes will be provided and should be striped for separate left and right turns.
- 6. Exiting site traffic should have Stop control.

Hotel and Casino Valet and Drop-off / Pick-Up Operations

As discussed, car valet attendants and the public dropping off patrons and luggage will enter via Canal Street then exit onto Potawatomi Circle. Valet attendants will drive past the truck loading dock and drive down the existing ramp to the underground parking. Hotel guest self-parkers will exit onto Potawatomi Circle and park in one of the parking garages. The reverse will occur for hotel pick-up.

Hotel bus guests will be dropped off at the entrance porte cochere and the bus will then drive to the east end where luggage will be taken into the hotel by staff. It is currently planned that hotel patrons leaving by bus will be picked up on the south side of the Bingo Casino.

The Bingo Casino valet drop-off / pick-up will take place in the existing west parking garage.

Parking

A new 480 stall parking garage will be built immediately east of the adjacent decks. The new garage will be interconnected to the other two garages. A one-bay surface parking lot will also be built, perhaps to be reserved for staff.

PART VI. TECHNICAL ADDENDUM

The following *Exhibits and Appendices* were previously referenced. They provide technical support for our observations, findings, and recommendations discussed in the text.

Exhibits

- 1. Site Vicinity Aerials
- 2. Photo Inventory
- 3. Existing Traffic
- 4. Site Plan
- 5. Project Traffic Characteristics
- 6. Site Traffic
- 7. Total Traffic
- 8. Intersection Capacity Analyses

Appendices

A. Capacity Analysis Worksheets

This Traffic Impact Analysis (TIA) for the proposed Potawatomi Bingo Casino Hotel prepared by:

Bin Grien

William C. Grieve, P.E., PTOE Senior Transportation Engineer GEWALT HAMILTON ASSOCIATES, INC.

bgrieve@gha-engineers.com



TECHNICAL ADDENDUM

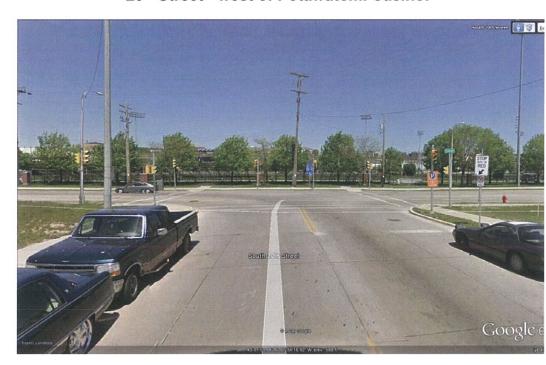
Exhibit 1 SITE LOCATION AERIALS







Aerial photograph of Canal Street at 20th Street - west of Potawatomi Casino.



Looking north along 20th Street across Canal Street.



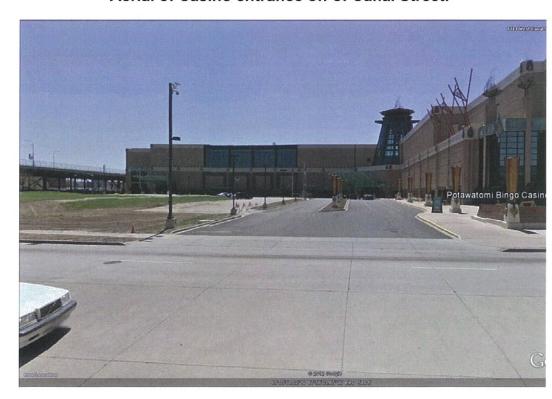
Looking eastbound along Canal Street across 20th Street.



Looking westbound along Canal Street across 20th Street.



Aerial of Casino entrance off of Canal Street.



Looking south towards Potawatomi Casino across Canal Street.



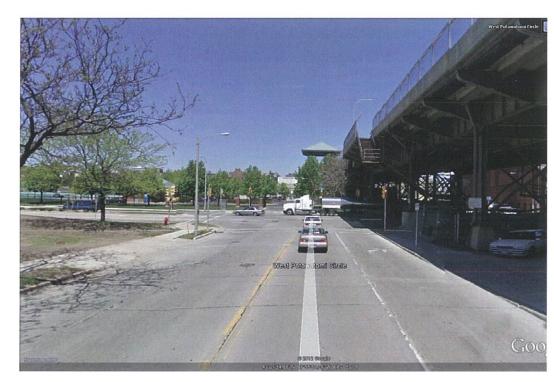
Looking eastbound along Canal Street across Casino Entrance.



Looking westbound along Canal Street across Casino entrance drive.



Aerial view of Canal Street at Potawatomi Circle.



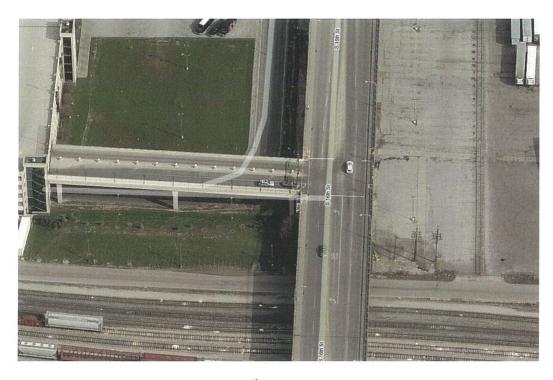
Looking northbound along Potawatomi Circle toward Canal Street.



Looking eastbound along Canal Street at Potawatomi Circle.



Looking westbound along Canal Street across Potawatomi Circle.



Aerial photograph of 16th Street at Potawatomi Casino parking deck entrance.



Looking westbound towards Potawatomi Casino Garage.



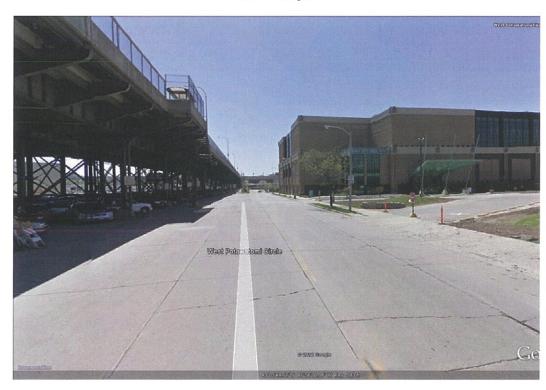
Looking northbound along 16th Street across parking deck entrance ramp.



Looking southbound along 16th Street across parking deck entrance ramps.



Looking northbound along Potawatomi Circle across Casino exit driveway.



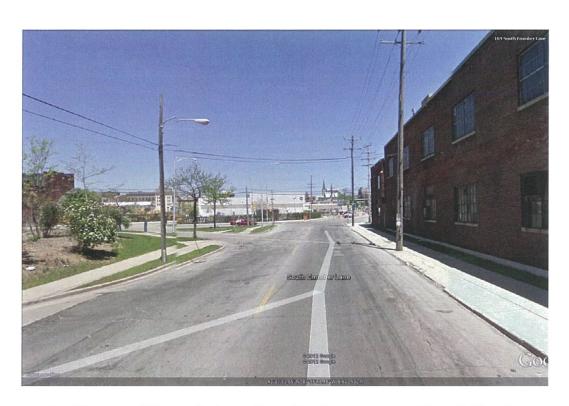
Looking southbound along Potawatomi Circle towards across Casino exit driveway.



Looking westbound toward Casino at the main exit driveway.



Looking westbound toward Casino at auxiliary exit driveway.



Looking northbound along Emmber Lane across Canal Street.



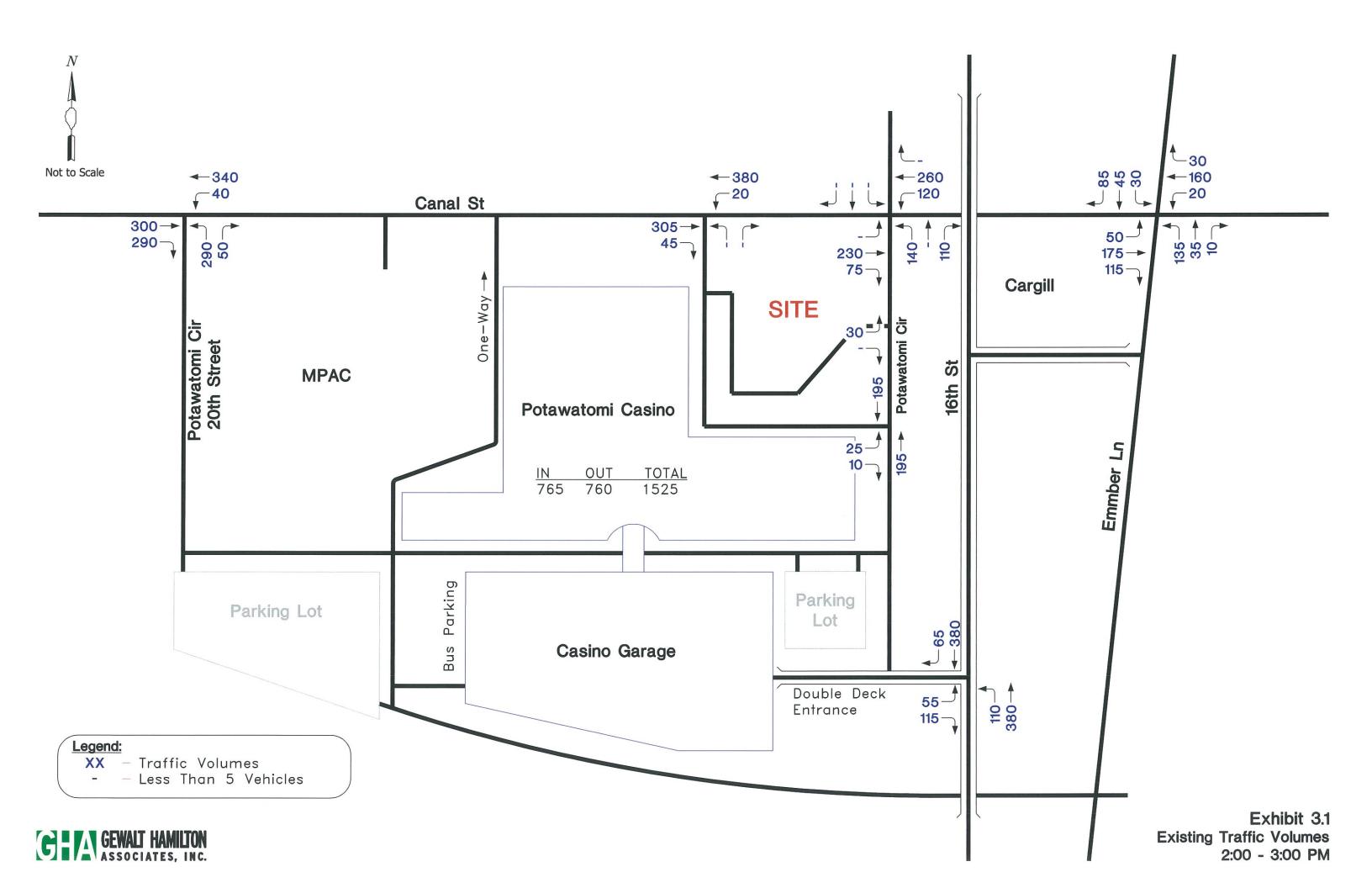
Looking southbound along Emmber Lane across Canal Street.

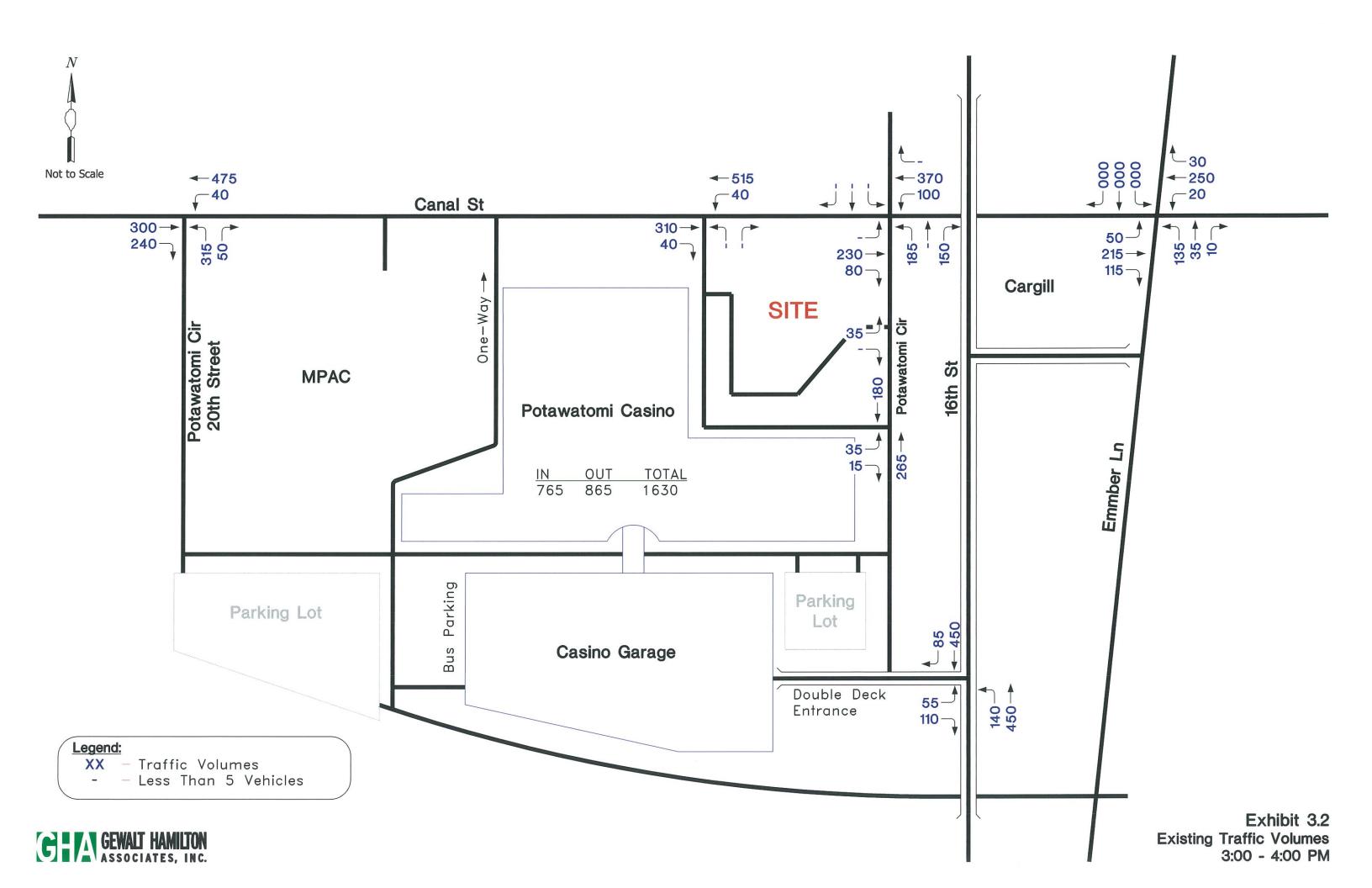


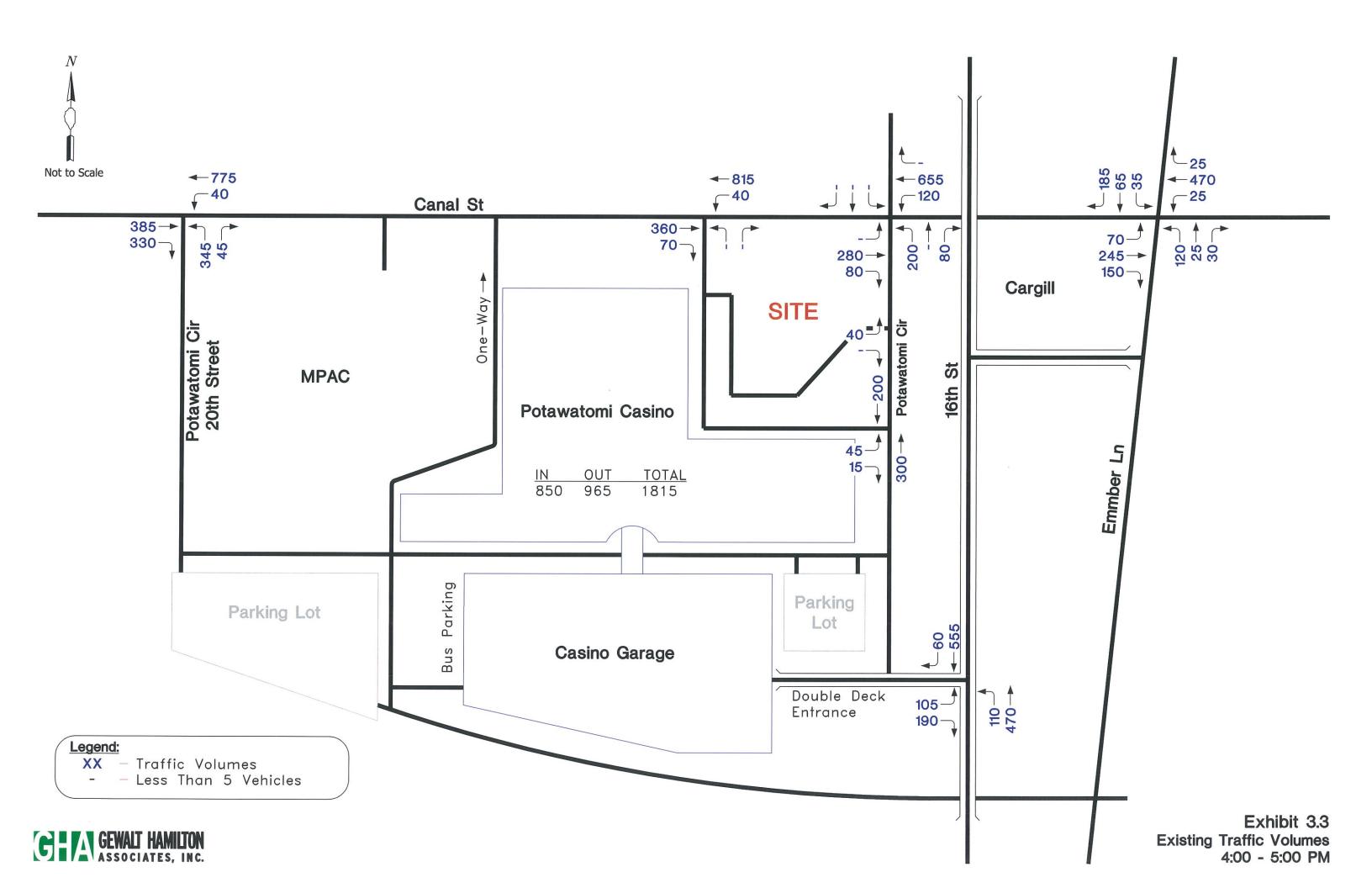
Looking eastbound along Canal Street across Emmber Lane.

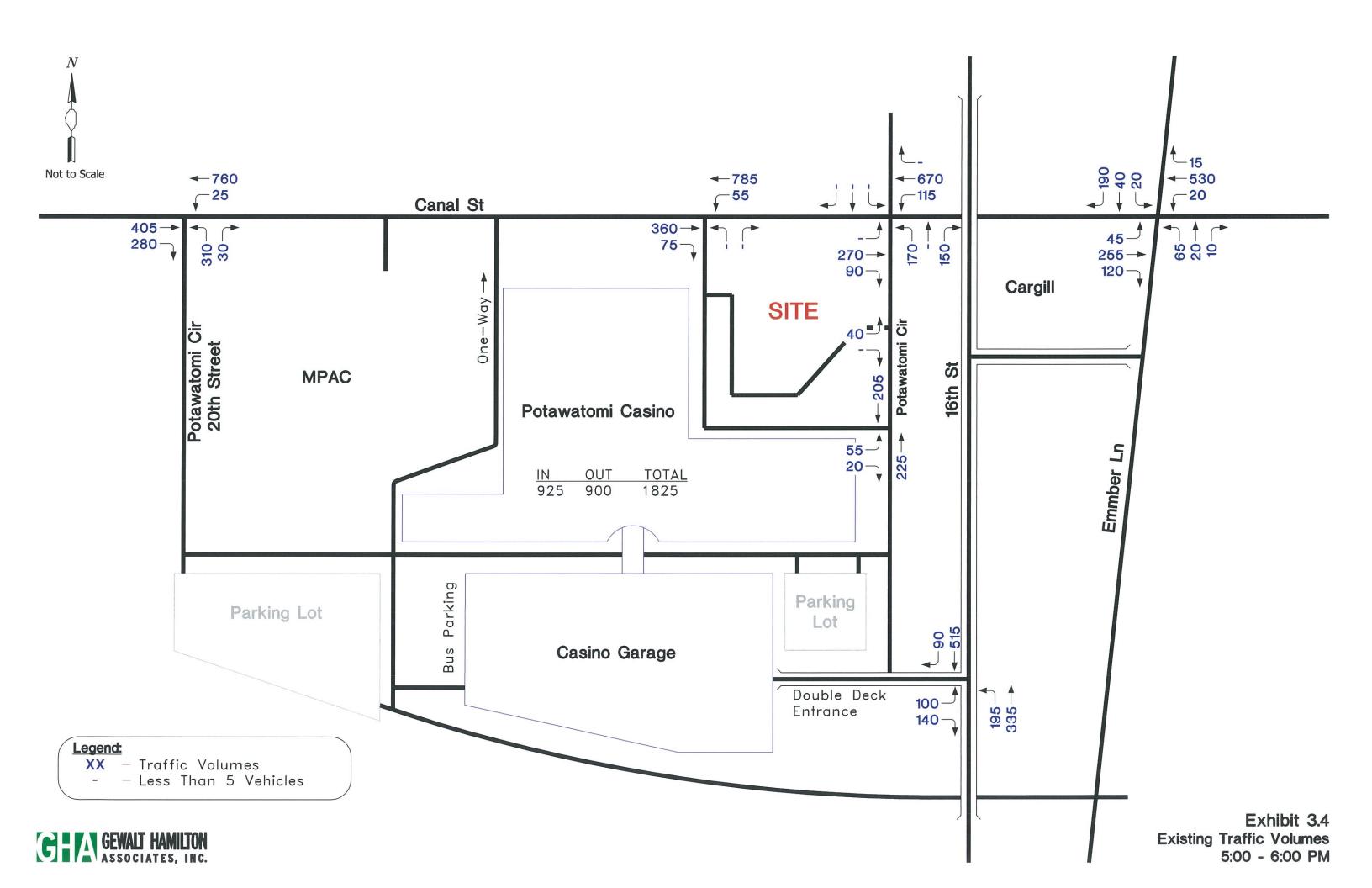


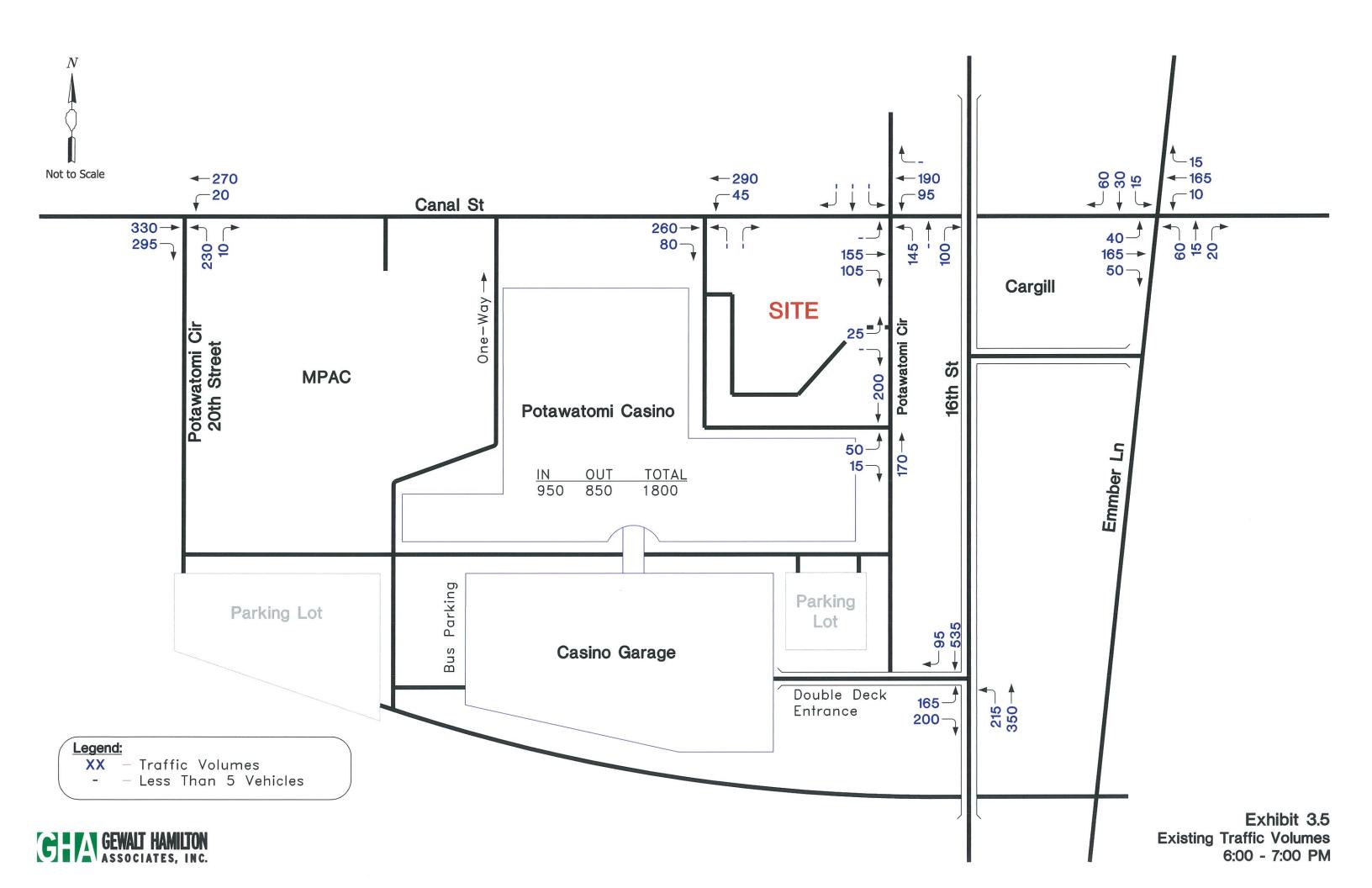
Looking westbound along Canal Street across Emmber Lane.

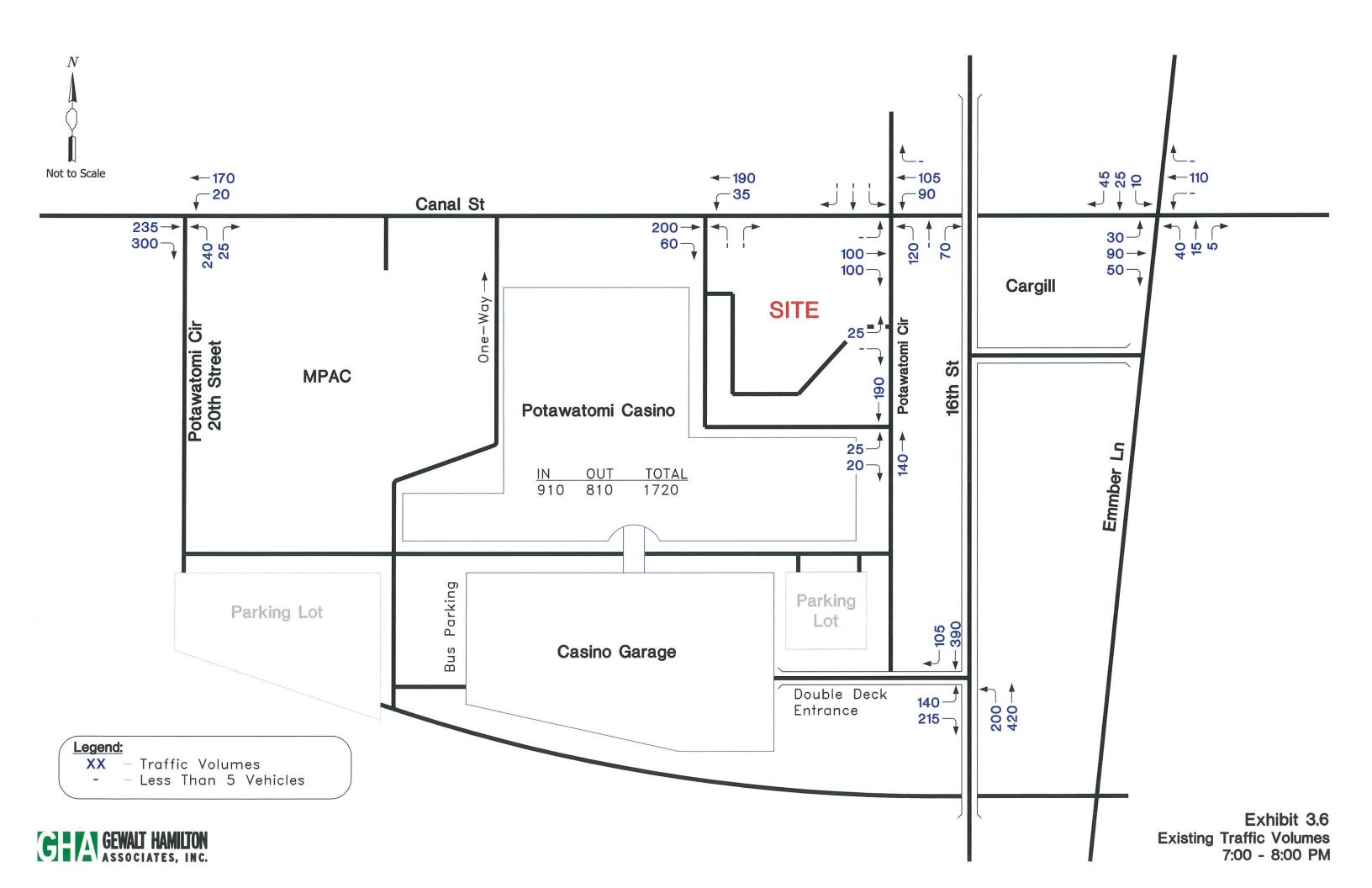


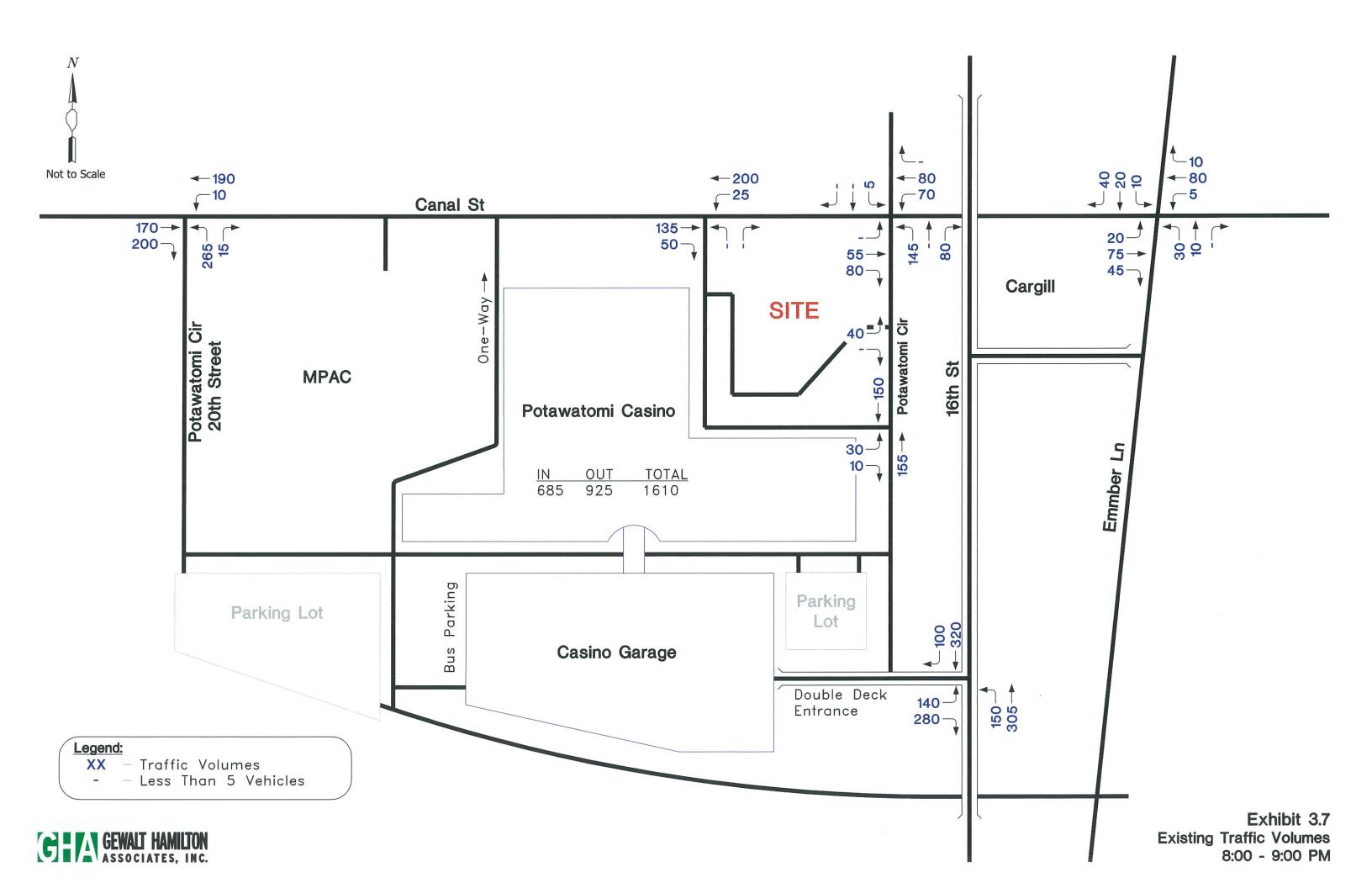












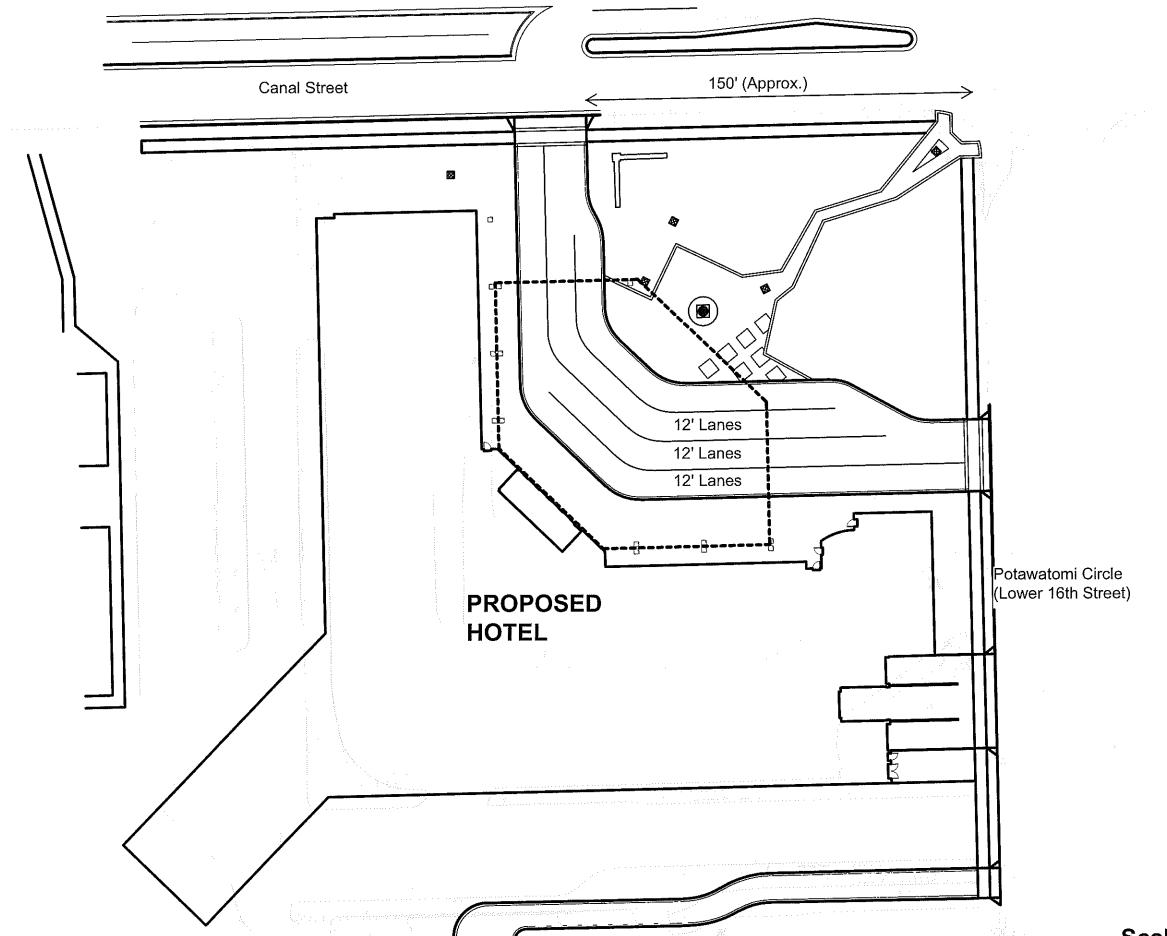


Exhibit 4
Site Plan

NORTH

Scale: 1"=40' (Approx.)

Exhibit 5 Project Traffic Characteristics

Potawatomi Bingo Casino Hotel - Milwaukee, Wisconsin

Part A. Traffic Generation Calculations

				Peak	Hours			Daily
	ITE	Wee	kday Mo	rning	Wee	kday Eve	ening	Weekday
	Code	In	Out	Sum	In	Out	Sum	Sum
Luxury Hotel	#310							
385 (rooms	140	80	220	120	110	230	3,070
Less trips on-site @ {	50% =	-70	-40	-110	-60	-55	-115	-1,535
	Totals =	70	40	110	60	55	115	1,535

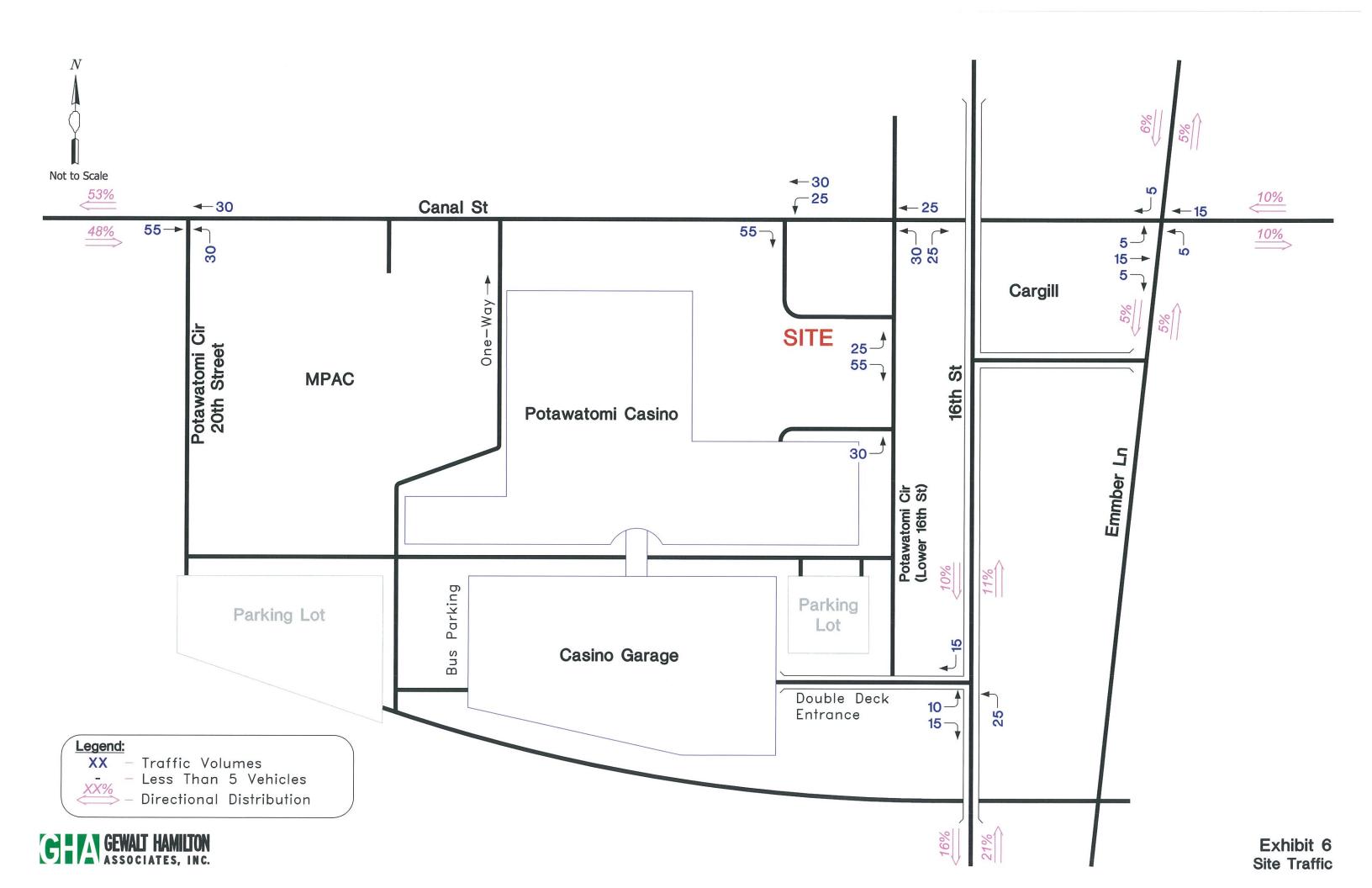
Notes:

- 1) Source: Institute of Transportation Engineers (ITE) Trip Generation Manual; 8th Edition
- 2) Per owner information, about 50% of hotel traffic would be already at the Bingo Casino. This discount was NOT taken to ensure the maximum hotel traffic impacts were tested.

Part B. Trip Distribution

		Percent	Use
Route & Direction		Approach Site From	Depart Site To
16th Street (Upper)			
 North of Garage Access 		10%	11%
- South of Garage Access		21%	16%
Emmber Lane			
 North of Canal Street 		6%	5%
 South of Canal Street 		5%	5%
Canal Street			
 East of Emmber Lane 		10%	10%
 West of Casino 		48%	53%
	Totals =	100%	100%





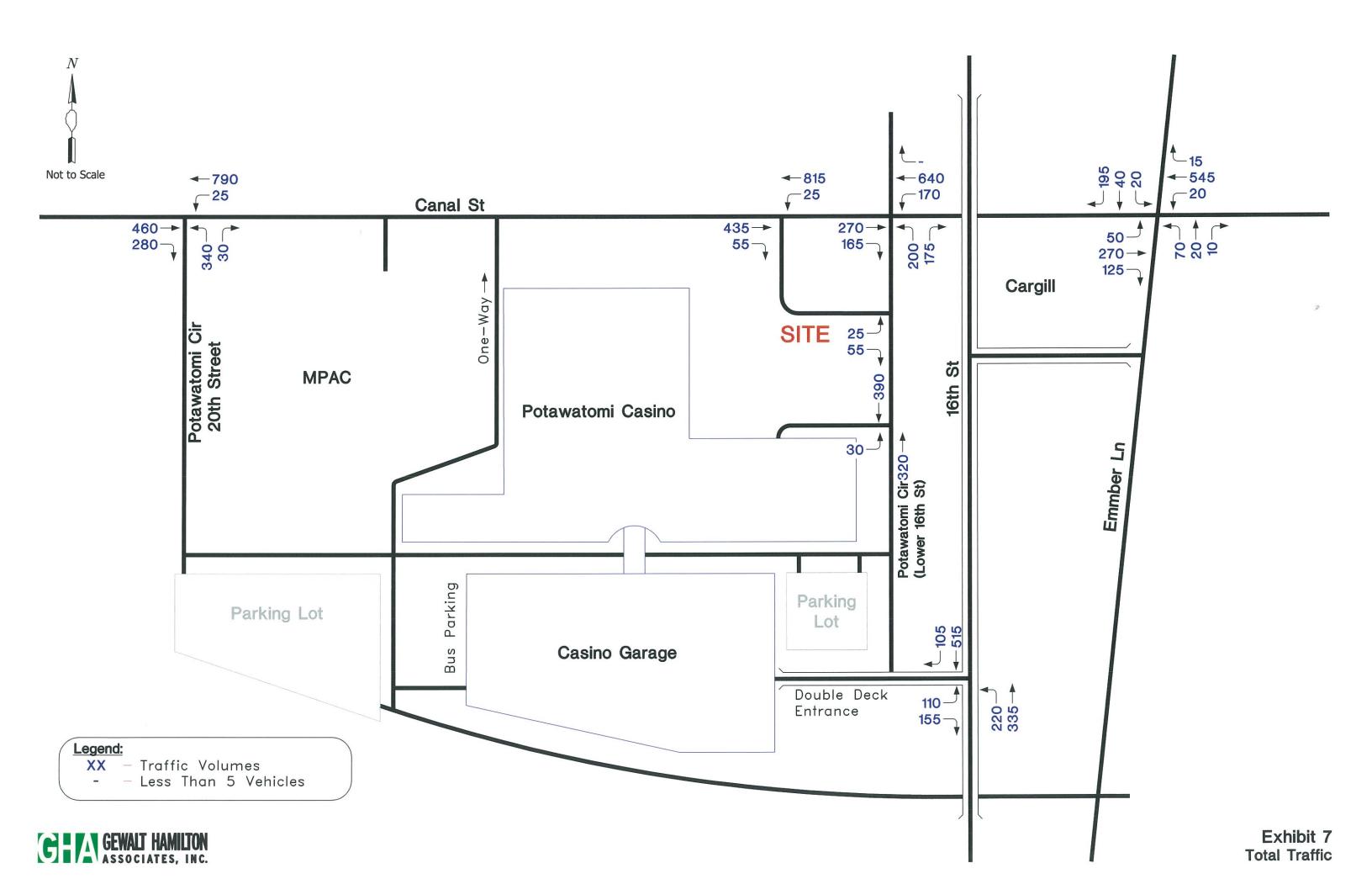


Exhibit 8 Intersection Capacity Analyses

Potawatomi Hotel; Milwaukee, Wisconsin

Part I. Parameters - Type of Traffic Control (Source: 2010 Highway Capacity Manual)

Current

Total Traffic

A. Traffic Signals												B. S	top	Sign	
LOS Delay (sec/veh)	Description											<u>LOS</u>	į	Delay (sec/vel	<u>n)</u>
A ≤ 10	All signal phases clear	waitin	g vehi	cles v	vithou	t delay						A		≤ 10	
B $> 10 \text{ and } \le 20$	Minimal delay experien	ced o	n sele	ct sigr	nal ph	ases						B		>10 and ≤ 15	
C >20 and ≤ 35	Some delay experience			35				desig	gn crite	eria		C		>15 and ≤ 25	
D >35 and \leq 55	Usually considered as				-							D		>25 and ≤ 35	
E >55 and \leq 80	Very long delays exper											E		>35 and ≤ 50	
F >80	Unacceptable delays e	xperie	nced	throug	jhout 1	he pea	ak ho	urs				F		>50	
Part II. Results	Intersection		(No			er Mo						ent)		Intersection Approac	
	Operations	East	oound	(EB)	West	bound	(WB)	North	bound	I (NB)	South	nbound	d (SB)	Delay	
	-	LT	TH	RT	LT	TH	RT		TH	RT	LT	TH	RT	(sec / veh)	LOS
	Traffic Signal													Intersection	Delay
Emmber Lane @ Canal Street		1,10													
Existing Traffic (see Exhibit 3)	 Current 	Α	A	<	В	В	<	>	В	<	>	В	<	15.1	В
Total Traffic (see Exhibit 7)	 Current 	Α	A	<	В	В	<	>	В	<	>	В	<	15.2	В
16th Street (lower) @ Canal Street														<u> </u>	
Existing Traffic	 Current 	-	A	<	>	В	-	В	-	В	_	2	_	11.0	В
		102	ft. = :	95th	% Q	ueue									
Total Traffic	Current	-	Α	<	>	В	-	В	-	В	-	-	_	11.4	В
		122	ft. = :	95th	% Q	ueue								50 ABS (100	
20th Street @ Canal Street		1000-00-00													
Existing Traffic	Current	_	A	<	>	В	_	>	C	<	_	_	_	10.7	В
Total Traffic	Current	_	A	<	>	В	_	>	C	<	_	_	_	11.3	В
16th Street (upper) @ Garage															
i siii sii soi (appoi) & daiage	1				1									1	1 1



9.7



APPENDIX A

INTERSECTION CAPACITY ANALYSES PRINTOUTS

Analyst: Bg Inter.: CANAL ST. @ EMMBER LN.

Agency: Area Type: All other areas

Date: 3/24/2012 Jurisd:

Period: PM PEAK Year : EXISTING

Project ID:

E/W St: CANAL STREET N/S St:

		SIGNA	LIZED I	NTERSEC	MOIT:	SUMMAR!	₹		
	Eastbou	nd 1	Westbour	nd		thbound	E	Southbo	:
	L T	R L	${f T}$	R	L	T I	₹	L T	R
No. Lanes LGConfig	1 2 L TR	· ·		0	0	1 (j	0 1 LT	
Volume Lane Width	45 255 12.0 12.0	120 20	530 .0 12.0	15		20 10 12.0) 2 	0 40 12.0	190
RTOR Vol		0		0	•	0	j		0
Duration	0.25	Area Typ	e: All o						
Phase Combi	nation 1	2	3 4	ļ		5	6	7	8
EB Left	A	A		ИВ	Left Thru	A A			
Thru Right	A A	A A		-	Right	A A			
Peds	11	21			Peds				
WB Left		A		SB	Left	A			
Thru		A			Thru	A			
Right Peds		A			Right Peds	A			
NB Right				l EB	Right				
SB Right				WB	Right				
Green	7.0	17.0			_	20.5			
Yellow	4.0	4.0				4.0			
All Red	0.0	1.5				2.0	- T	. <u>-</u> 60 0	
	т	ntersecti	on Perfo	ormance	e Summa	_	a rend	th: 60.0	secs
Appr/ Lar		j Sat	Ratios			Group	Appr	oach	
Lane Gro	nun Elo	w Rate _			rane (O = O = E	~ ~		
(1200									
Grp Car	pacity		/c g,	7c	Delay		Delay	LOS	
Eastbound	pacity	(s) v			Delay	LOS		LOS	
Eastbound L 40	pacity 	(s) v 05 0	.12 0	. 49	Delay			LOS	
Eastbound L 40 TR 16	pacity	(s) v 05 0	.12 0		Delay	LOS	Delay	A A A A A A A A A A A A A A A A A A A	
Eastbound L 40 TR 16	Dacity	05 0 44 0	.12 0 .25 0	. 49 . 47	Delay 8.8 9.7	LOS A A	Delay	A A A A A A A A A A A A A A A A A A A	
Eastbound L 40 TR 16 Westbound L 28	Dacity 03 18 507 34	05 0 44 0	.12 0	. 49 . 47	Delay	LOS	Delay	A A A A A A A A A A A A A A A A A A A	
Eastbound L 40 TR 16 Westbound L 28	Dacity 03 18 507 34	05 0 44 0	.12 0 .25 0	. 49 . 47	Delay 8.8 9.7	LOS A A	Delay	A	
Eastbound L 40 TR 16 Westbound L 28 TR 10	Dacity 03 18 507 34 30 98 021 36	05 0 44 0 8 0 02 0	.12 0 .25 0	. 49 . 47 . 28 . 28	Delay 8.8 9.7	LOS A A B B	Delay	A	
Eastbound L 40 TR 16 Westbound L 28 TR 10	Dacity 03 18 507 34 30 98 021 36	05 0 44 0 8 0 02 0	.12 0 .25 0	. 49 . 47 . 28 . 28	Delay 8.8 9.7 15.9	LOS A A B B	9.6 18.9	В	
Eastbound L 40 TR 16 Westbound L 28 TR 10 Northbound LTR 45	Dacity 03 18 507 34 80 98 021 36	(s) v 05 0 44 0 8 0 02 0	.12 0 .25 0	. 49 . 47 . 28 . 28	8.8 9.7 15.9 19.0	LOS A A B B B	9.6 18.9	В	

Analyst: Bg Inter.: CANAL ST. @ EMMBER LN.

Agency: Area Type: All other areas

Date: 3/24/2012 Jurisd:

Period: PM PEAK Year : TOTAL

Project ID:

E/W St: CANAL STREET N/S St: 50

				SI	GNALI	ZED II	TERSE	CTION	SUMMAF	₹Y			
		Eas	tbour			stbour			thbour		Sou	thbound	
]	L	Т	R	L 	T	R	L	${f T}$	R	L	T R	
No. Lan	ıes	1	2	0	1	2	0	0	1	0	0	1 0	i
LGConfi	g	L	TR		L	TR			LTR			LTR	
Volume		50	265	125	20	545	15			LO	20	40 19	5
Lane Wi		12.0	12.0	^	12.0	12.0	0		12.0	, !		12.0	
RTOR Vo	.———— ьт l			0	l 		0		·	·		0	I
Duratio	n	0.25		Area '			ther perat						
Phase C	ombin	atior	1 1	2	3	4	<u></u>		5	6	7	8	
EB Lef	t		A	A			NB	Left	A				
Thr			Α	A				Thru	A				
Rig			A	A				Right	A				
Ped								Peds	_				
WB Lef				A			SB	Left	A				
Thr				A			-	Thru	A				
Rig				A			-	Right	. A				
Ped							===	Peds					
NB Rig							EB	Right					
SB Rig	ınt			17 0			WB	Right	20.5				
Green			7.0	17.0					$\frac{20.5}{4.0}$				
Yellow All Red	1		4.0	4.0 1.5					2.0				
AII Ked	l		0.0	1.5						le Len	gth:	60.0	secs
								e Summ					
Appr/	Lane		_	j Sat		atios		Lane	Group	App	roach	1	
Lane	Grou			v Rate									
Grp	Capa	city	l	(s)	v/c	g,	C'C	Delay	LOS	Dela	у гоз	3	
Eastbou							4.0						
L	396		180		0.13		. 49	8.9	A	0 5	*		
TR	160)'/	344	13	0.26	5 0.	. 47	9.8	A	9.7	A		
Westbou	ınd												
L,	275	•	972	2	0.08	30.	. 28	15.9	В				
TR	102	:1	360)3	0.58	3 0.	. 28	19.2	В	19.1	. В		
Northbo	und												
LTR	446	5	130	06	0.24	1 0.	. 34	14.4	В	14.4	В		
Southbo	und												
		1	161	: 0	0.4	7 0	. 34	16.1	Œ	16.1	В		
LTR	567			59									
	Int	ersec	ction	Delay	= 15	. 2 (s	sec/ve	h) I	nterse	ection	LOS	= B	

Inter.: CANAL ST. @ 16th ST. (lower) Analyst: Bg

Area Type: All other areas Agency: Jurisd:

Date: 3/24/2012

Period: PM PEAK Year : EXISTING

Project ID:

E/W St: CANAL STREET N/S St: 16th STREET (lower)

					SI	GNALI	ZED IN	TERSE	CTION	SUMM	ARY				
			Eas	stboui	nd	We:	stbour	ıd	Nor	thbou	ınd	Sou	thbou	ınd	
			L	T	R	L	${f T}$	R	L	Т	R	L	${f T}$	R	
LGC Vol Lan	Lane onfig ume e Wid R Vol) lth	 0 	2 TR 270 12.0	0 90 0	1115	2 LT 670 12.0	0	1 L 170 12.0	0	1 R 150 12.0 0	0	0	0	
 Dur	atior	 1	0.25		Area										
	0			_ 1			_)perat	ions	 5	6		8		
Pna EB	se Co Left		natio	n I	2	3	4	NB	Left	a A	ь	/	a	i	
CD	Thru			A				IND	Thru	A					
	Righ			A				i	Right	A					
	Peds			**					Peds						
WB	Left			A				SB	Left						
	Thru	1		A					Thru						
	Righ	ıt							Right						
	Peds								Peds						
NB	Righ							EB	Right						
SB	Righ	ıt						WB	Right						
Gre				31.0						18.0	}				
	low Red			4.0 1.5						3.5					
FY 7. 1.	Keu			1.0							cle Len	ath:	60.0	s	ecs
				Iı	nterse	ction	Perfo	rmanc	e Summ	-		9 0			
App	<u>r/</u>	Lane	======================================		j Sat		atios		Lane			roach			
Lan		Grou	ıp	Flo	w Rate	-									
Grp		Capa	acity		(s)	v/c	g/	'C	Delay	LOS	Dela	y LOS			
Eas	tbour	nd													
TR		179	9	348	32	0.2	1 0.	52	7.9	A	7.9	А			
Wes	tbour	nd													
LT		153	32	296	55	0.5	40.	52	10.1	В	10.1	В			
Nor	thbou	ınd													
L		542	2	180	05	0.3	30.	30	16.7	В	16.7	В			
R Sou	thbou	489 and	5	16:	1.5	0.3	30.	30	16.7	В	10.7	Q.			

Intersection Delay = 11.0 (sec/veh) Intersection LOS = B

Analyst: Bg Inter.: CANAL ST. @ 16th ST. (lower)

Agency:

Area Type: All other areas

Jurisd:

Date: 3/24/2012

Year : TOTAL

Period: PM PEAK

Project ID: E/W St: CANAL STREET

N/S St: 16th STREET (lower)

	Ea	stbou	nd	We	stbou	nd	Nor	thbo	und	Sot	ıthbo	und
	L	${f T}$	R	L	${f T}$	R	L	${f T}$	R	L	T	R
No. Lanes	0	2	0	0	2	0	- - 	0	1	0	0	0
LGConfig		$_{ m LT}$	R		$_{ m LT}$	R	L		R			
Volume	0	270	165	170	640	0	170		150			
Lane Width	j	12.0			12.0		12.0		12.0	Ì		
RTOR Vol	j		0	į		0	Ì		0	j		

Dur	ation	0.25		Area	Турє	: Al	l ot	her	areas				
					S	igna	ıl Or	perat	ions				
Pha	se Combi	nation	1	2	3	,	4			5	6 7	8	1
EB	Left		A					NB	Left	A			
	Thru		A						Thru	A			
	Right		A						Right	A			
	Peds								Peds				
WB	Left		A					SB	Left	A			
	Thru		Α						Thru	A			
	Right		A						Right	A			
	Peds								Peds				
NB	Right							EB	Right				
SB	Right							WB	Right				
Gre	en		31.0							18.0			
Yel	low		4.0							3.5			
All	Red		1.5							2.0			
										Cycle	Length:	60.0	secs

		Intersec	tion P	erforman	ce Summa	ıry		
Appr/ Lane	Lane Group	Adj Sat Flow Rate	Rat	ios	Lane G	roup	Appro	oach
Grp	Capacity		v/c	g/C	Delay	LOS	Delay	LOS
Eastbou	ınd							
LTR	1762	3411	0.26	0.52	8.2	A	8.2	A
Westbou	ınd							
LTR	1394	2699	0.61	0.52	11.0	В	11.0	В
Northbo	ound							
L	542	1805	0.33	0.30	16.7	В	16.7	В
R Southbo	485 ound	1615	0.33	0.30	16.7	В		_

Intersection Delay = 11.4 (sec/veh) Intersection LOS = B

Inter.: CANAL ST. @ 20th ST. Analyst: Bg

Area Type: All other areas Agency: Date: 3/24/2012

Jurisd:

Year : EXISTING Period: PM PEAK

Project ID:

E/W St: CANAL STREET N/S St: 20th STREET

E/W SC: CAN	WI SIK	E) E) T				14 / 13	SC. 2	ocn bi	1(1)11				
MALL COLOR PLANT AND THE PARTY OF THE PARTY	Eas	tbound			ZED IN stboun		CTION Nor	SUMMAR thboun		Sout	hbou	 .nd	
	L	T	R	L	T	R	L	${f T}$	R	L	Т	R	j
No. Lanes LGConfig Volume Lane Width	1	2 TR 405 2	0	0	2 LT 760 12.0	0	1 L 310 12.0		0	0	0	0	
RTOR Vol		C) į				İ	0	j				j
Duration	0.25	Z	Area T		All o		areas ions		**************************************				
Phase Combi EB Left Thru Right	nation	1 A A	2	3	4	 NB 	Left Thru Right	5 A A	6	7	8		4444
Peds WB Left Thru Right Peds NB Right		A A				 SB EB	Peds Left Thru Right Peds Right						
SB Right Green Yellow All Red	•	32.5 4.0 1.5				WB	Right	17.0 3.5 1.5 Cycl	e Leng	ıth: 6	50.0	ŝ	secs
Appr/ Lar		Int Adj			Perio atios	rmanc	e Summ Lane	ary Group		roach		· · · · · · · · · · · · · · · · · · ·	
Lane Gro Grp Cap	oup pacity	Flow (s	Rate s)	v/c	g/	_ C	 Delay	LOS	Delay	LOS			
Eastbound		****											
TR 18	340	3396	5	0.39	9 0.	54	8.1	A	8.1	А			
Westbound													
LT 18	304	3330)	0.40	5 0.	54	8.6	А	8.6	A			
Northbound L 51 LR 45	L1	1805 1615		0.64		28 28	21.5 15.8	C B	21.0	С			
Southbound													

Southbound

Intersection Delay = 10.7 (sec/veh) Intersection LOS = B

Analyst: Bg Inter.: CANAL ST. @ 20th ST. Agency: Area Type: All other areas

Date: 3/24/2012 Jurisd:

Period: PM PEAK Year : TOTAL

Project ID:

E/W St: CANAL STREET N/S St: 20th STREET

			ST	CNAT.T	ZED TN	JUERSE	CTION	STIMMA	RY				
	Eas	stbou			stbour		_	thbou		Soi	ıthbo	und	1
	L	${f T}$	R	L	T	R	L	T	R	L	T	R	İ
No. Lanes	0	2	0	0		0	1 1	0	0	0	0	0	
LGConfig		TR			LT 760		L 340	LR	30				
Volume Lane Width	-	460 12.0	280	25	12.0		112.0	12 ກ	30				ł
RTOR Vol		12.0	0		12.0		12.0	12.0	0				
Duration	0.25		Area				areas					· ————	
Discount Courts					gnal (4	Dperat	ions	5	6	<u>-</u>		8	
Phase Combi	Lnation	л т	4	3	4	 NB	Left	a A	0	/		0	
Thru		A				1412	Thru	7					
Right		A					Right	A					
Peds		••				i	Peds						
WB Left		A				SB	Left						
Thru		A				İ	Thru						
Right						į	Right						
Peds							Peds						
NB Right						EB	Right						
SB Right						WB	Right						
Green		32.5						17.0)				
Yellow		4.0						3.5					
All Red		1.5						1.5					
		717	~ + o ~ a o	ation	Donf	~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	e Summ		cle Ler	igth:	60.0		sec
Appr/ Lar		Ađ	j Sat	R	atios	JIMAIIC	Lane		Apr	roacl	<u>a</u>		
Lane Gro	oup pacity		w Rate (s)	v/c	g,	/C	Delay	LOS	Dela	y LOS	 3		
Eastbound													 -
TR 18	348	၁ /I	12	0.4	2 0	. 54	8.3	А	8.3	А			
TK 10	040	34	.J., Z4	0.4	<u>د</u> 0,	.54	0.3	A	0.3	A			
Westbound													
LT 17	798	33	20	0.4	6 0	.54	8.6	A	8.6	A			
Northbound						0.0							
L 51	L1	18	05	0.7	υ Ο.	.28	23.5	С					

Southbound

458

1615

LR

Intersection Delay = 11.3 (sec/veh) Intersection LOS = B

0.07 0.28

15.8 B 22.9 C

Jurisd:

Analyst: Bg Inter.: 16th STREET (upper) @ GARAGE

Agency: Area Type: All other areas

Date: 3/24/2012

Period: PM PEAK Year : EXISTING

Project ID:

E/W St: GARAGE N/S St: 16th STREET (upper)

Eastbound L T R L T R L T R L T R L T R C Config L R L T R L T R L T R C Config L R L T R L T R C Config L R L T R C Config L R L T T R C Config L R L T T R C Config L R L T T R C Config L R L T T R C Config L R L T T R C Config L R L T T R C Config L R L T T R C Config L R L T T R C Config L R L T T R C Config L R L T T R C Config L R L T L T T R C Config L R L T
O. Lanes 1
Config
Signal Operations
Combination 1 2 3 4
NB
Right A Peds B Left Thru Right A Peds B Left Thru Right A Peds B Left Thru Right A Right A Right A Peds B Right Peds B Right B Right B Right Freen 17.0 33.0 ellow 3.5 11 Red 1.5 1.5 Intersection Performance Summary ppr/ Lane Adj Sat Ratios Lane Group Approach ane Group Flow Rate rp Capacity (s) v/c g/C Delay LOS astbound 511 1805 0.21 0.28 16.6 B 458 1615 0.32 0.28 17.4 B estbound orthbound orthbound 414 753 0.50 0.55 9.3 A
Peds Peds
B Left Thru
Thru Right Peds Right Peds Right Rig
Right Peds Right Peds Right Ri
Peds B Right B Right B Right Creen Creen Creen Crycle Length: 60.0 secs
EB Right B Right WB Right WB Right Reen 17.0 33.0 3.5 3.5 1.5 Cycle Length: 60.0 Secs Cy
B Right reen 17.0 33.0 ellow 3.5 11 Red 1.5 27cle Length: 60.0 secs Intersection Performance Summary ppr/ Lane Adj Sat Ratios Lane Group Approach ane Group Flow Rate rp Capacity (s) v/c g/C Delay LOS Delay LOS astbound 511 1805 0.21 0.28 16.6 B 458 1615 0.32 0.28 17.4 B estbound orthbound 414 753 0.50 0.55 9.3 A
reen 17.0 33.0 ellow 3.5 3.5 11 Red 1.5 Cycle Length: 60.0 secs
1.5 Cycle Length: 60.0 secs
Cycle Length: 60.0 secs
ppr/ Lane Adj Sat Ratios Lane Group Approach ane Group Flow Rate rp Capacity (s) v/c g/C Delay LOS Delay LOS astbound 511 1805 0.21 0.28 16.6 B 17.0 B 17.0 B estbound 6 414 753 0.50 0.55 9.3 A
ane Group Flow Rate rp Capacity (s) v/c g/C Delay LOS Delay LOS astbound 511 1805 0.21 0.28 16.6 B 17.0 B 458 1615 0.32 0.28 17.4 B estbound orthbound 414 753 0.50 0.55 9.3 A
rp Capacity (s) v/c g/C Delay LOS Delay LOS astbound 511 1805 0.21 0.28 16.6 B 17.0 B 458 1615 0.32 0.28 17.4 B estbound orthbound 414 753 0.50 0.55 9.3 A
astbound 511 1805 0.21 0.28 16.6 B 17.0 B 458 1615 0.32 0.28 17.4 B estbound orthbound 414 753 0.50 0.55 9.3 A
511 1805 0.21 0.28 16.6 B 17.0 B 458 1615 0.32 0.28 17.4 B estbound orthbound 414 753 0.50 0.55 9.3 A
17.0 B 458 1615 0.32 0.28 17.4 B estbound forthbound 414 753 0.50 0.55 9.3 A
458 1615 0.32 0.28 17.4 B estbound orthbound 414 753 0.50 0.55 9.3 A
estbound Orthbound 414 753 0.50 0.55 9.3 A
orthbound 414 753 0.50 0.55 9.3 A
414 753 0.50 0.55 9.3 A
414 753 0.50 0.55 9.3 A
414 753 0.50 0.55 9.3 A
1990 9010 0.10 0.00 14 7.7
outhbound
R 1945 3537 0.33 0.55 7.5 A 7.5 A
Intersection Delay = 9.2 (sec/veh) Intersection LOS = A

Jurisd:

Analyst: Bg Inter.: 16th STREET (upper) @ GARAGE

Agency: Area Type: All other areas

Date: 3/24/2012

Period: PM PEAK Year : TOTAL

Project ID:

E/W St: GARAGE N/S St: 16th STREET (upper)

			SIG	NALIZ	ED TI	ILEKSE	CTION	SUMMA	7K X				
	Eastbound			Westbound			Northbound			Southbound			ļ
	L	Т	R	L	${f T}$	R	L 	${f T}$	R	L	${f T}$	R	
No. Lanes	1	0	1	0	0	0	1	2	0	0	2	0	-
LGConfig	L		R				L	${f T}$			TR		ļ
Volume	110		155				220	335			515	105	ļ
Lane Width	12.0		12.0				12.0	12.0			12.0		ļ
RTOR Vol			0									0	l
Duration	0.25		Area T				areas ions						
Phase Combi	nation	1	2	3	4			5	6	7		8	
EB Left		Α				NB	Left	A					
Thru							Thru	A					
Right		Α					Right	: A					
Peds							Peds						
WB Left] SB	Left						
Thru							Thru	А					
Right							Right	: A					
Peds						İ	Peds						
NB Right						EB	Right						
-													
ss kiant						WB	Right	-					
-		17.0				WB	Right	: 33.0)				
Green						WB	Right	33.0)				
Green Yellow		3.5				WB	Right)				
Green Yellow		3.5 1.5			_	,		33.0 3.5 1.5 Cyc) cle Ler	ngth:	60.0	se	ecs
Green Yellow All Red		3.5 1.5 Ir	ntersec i Sat			,	e Summ	33.0 3.5 1.5 Cyc nary	cle Ler			se	∋C\$
Green Yellow All Red Appr/ Lan	 1e	3.5 1.5 Ir Ad	j Sat		Perfo tios	,		33.0 3.5 1.5 Cyc nary	cle Ler	ngth: oroac		S 6	ec:
Green Yellow All Red Appr/ Lan Lane Gro	 1e	3.5 1.5 Ir Ad: Flow				ormanc	e Summ	33.0 3.5 1.5 Cyc ary Group	cle Ler		h	se	ecs
Green Yellow All Red Appr/ Lan Lane Gro Grp Cap	 ne oup	3.5 1.5 Ir Ad: Flow	j Sat w Rate	Ra	tios	ormanc	e Summ Lane	33.0 3.5 1.5 Cyc ary Group	cle Ler	roac	h	86	∋C:
Green Yellow All Red Appr/ Lan Lane Gro Grp Cap	ne oup oacity	3.5 1.5 Ir Ad: Flow	j Sat w Rate (s)	Ra	tios g,	ormanc	e Summ Lane	33.0 3.5 1.5 Cyc ary Group	cle Ler	roac	h	s e	∋C :
Green Yellow All Red Appr/ Lan Lane Gro Grp Cap	ne oup oacity	3.5 1.5 Ir Ad: Flow	j Sat w Rate (s)	Ra v/c	tios g,	ormanc /C	e Summ Lane ——— Delay	33.0 3.5 1.5 Cycury_ Group	cle Ler	oroac	h S	se	ec:
Green Yellow All Red Appr/ Lan Lane Gro Grp Cap Eastbound L 51	ne oup oacity	3.5 1.5 Ir Ad: Flow	j Sat w Rate (s) 	Ra v/c	tios g/ 0.	ormanc /C	e Summ Lane ——— Delay	33.0 3.5 1.5 Cycury_ Group	ole Ler Dela	oroac	h S	S 6	⊖C:
Green Yellow All Red Appr/ Lan Lane Gro Grp Cap Eastbound L 51	ne oup oacity	3.5 1.5 —Ir Adj Flow	j Sat w Rate (s) 	v/c 0.23	tios g/ 0.	ormanc /C	e Summ Lane ———— Delay	33.0 3.5 1.5 Cycuary_ Group LOS	ole Ler Dela	oroac	h S	86	ec:
Green Yellow All Red Appr/ Lan Lane Gro Grp Cap Eastbound L 51	ne oup oacity	3.5 1.5 —Ir Adj Flow	j Sat w Rate (s) 	v/c 0.23	tios g/ 0.	ormanc /C	e Summ Lane ———— Delay	33.0 3.5 1.5 Cycuary_ Group LOS	ole Ler Dela	oroac	h S	se	⊖C;
Green Yellow All Red Appr/ Lan Lane Gro Grp Cap Eastbound L 51 R 45 Westbound	ne oup oacity	3.5 1.5 —Ir Adj Flow	j Sat w Rate (s) 	v/c 0.23	tios g/ 0.	ormanc /C	e Summ Lane ———— Delay	33.0 3.5 1.5 Cycuary_ Group LOS	ole Ler Dela	oroac	h S	se	ec:
Green Yellow All Red Appr/ Lan Lane Gro Grp Cap Eastbound L 51 R 45 Westbound	ne oup pacity .1	3.5 1.5 In Adgree Adgre	j Sat w Rate (s) 05	0.23	g, 0.	ormanc /C .28	Delay	33.0 3.5 1.5 Cycury_ Group LOS B	ole Ler Dela	oroac	h S	86	∋c:
Green Yellow All Red Appr/ Lan Lane Gro Grp Cap Eastbound L 51 R 45 Westbound Northbound L 40	oup pacity .1	3.5 1.5 —Ir Ad: Flow 180 163	j Sat w Rate (s) 	0.23 0.36	g, 0.	ormanc /C .28	Delay	33.0 3.5 1.5 Cycurry_ Group TLOS B B	Dela	proac	h S	86	∋c:
Green Yellow All Red Appr/ Lan Lane Gro Grp Cap Eastbound L 51 R 45 Westbound Northbound L 40 T 19	oup pacity .1	3.5 1.5 —Ir Ad: Flow 180 163	j Sat w Rate (s) 05 15	0.23 0.36	g, 0.	ormanc /C .28	Delay	33.0 3.5 1.5 Cycurry_ Group TLOS B B	Dela	proac	h S	86	⊖C:
Green Yellow All Red Appr/ Lan Lane Gro Grp Cap Eastbound L 51 R 45 Westbound Northbound L 40 T 19 Southbound	ne oup pacity .1 .8	3.5 1.5 In Add Flow 180 163	j Sat w Rate (s) 05 15	0.23 0.36	0.000	ormanc /C .28 .28	16.7 17.6	33.0 3.5 1.5 Cycury_ Group LOS B B	Dela	proac	h S	se	∋C;
Green Yellow All Red Appr/ Lan Lane Gro Grp Cap Eastbound L 51 R 45 Westbound Northbound L 40 T 19 Southbound	ne oup pacity .1 .8	3.5 1.5 In Add Flow 180 163	j Sat w Rate (s) 05 15	0.23 0.36	0.000	ormanc /C .28 .28	16.7 17.6	33.0 3.5 1.5 Cycury_ Group LOS B B	Dela	proac	h S	se	⇒C;