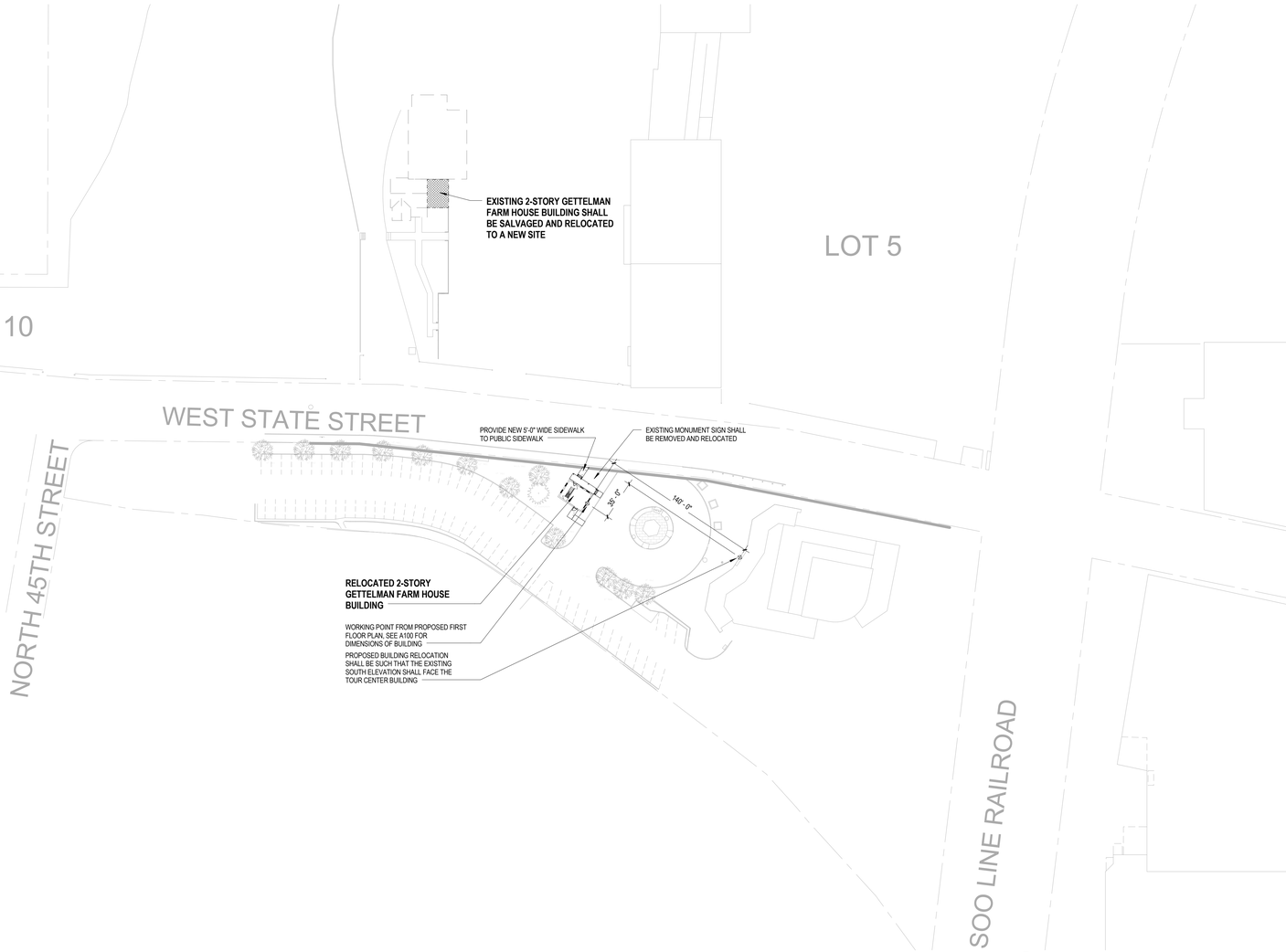


1 ARCHITECTURAL SITE RELOCATION PLAN
1" = 50'-0"

GENERAL NOTES - BUILDING RELOCATION	
1.	MAIN GOAL OF THIS PROJECT WILL BE TO SUCCESSFULLY RELOCATE THE STRUCTURE ACROSS STATE STREET TO AN AREA WHERE THE BUILDING CAN BE BETTER ENJOYED BY THE PUBLIC.
2.	NOTIFY ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES WITH EXISTING CONDITIONS AND/OR DIMENSIONS PRIOR TO PROCEEDING WITH WORK.
3.	PROVIDE AND INSTALL ALL EQUIPMENT, SHORING AND/OR BRACING NECESSARY TO SECURE BUILDING FOR A MOVE, INCLUDING EXCAVATION REQUIRED FOR ANY SUPPORT STRUCTURE INSTALLATION.
4.	PROVIDE AMPLE PROTECTION OF THE BUILDING TO PREVENT DAMAGE DURING THE MOVE.
5.	SECURE PERMITS FOR MOVING THE BUILDING, INCLUDING BUT NOT LIMITED TO ANY STREET CLOSURE PERMITS.
6.	VERIFY ANY HEIGHT RESTRICTIONS TO THE MOVE ACROSS STATE STREET. ELECTRIC, TELEPHONE, CABLE, ETC. LINES THAT WOULD HAVE TO BE LIFTED OR TEMPORARILY DISCONNECTED TO FACILITATE THE MOVE. COST FOR DISCONNECT SHALL BE INCLUDED IN THE SCOPE OF WORK.
7.	BUILDING SHALL BE RELOCATED TO A NEW FOUNDATION (POURED IN-PLACE CONCRETE BASEMENT WALLS). COORDINATE ANY FOUNDATION DESIGN DETAILS WITH ARCHITECT AND STRUCTURAL ENGINEER TO FACILITATE THE REMOVAL OF ANY SUPPORT STRUCTURE USED FOR THE BUILDING MOVE.
8.	MOVING CONTRACTOR IS RESPONSIBLE FOR REMOVAL OF ALL WASTE GENERATED FROM THEIR PROCESS.

SITE REFERENCE PLAN
NTS

DESCRIPTION	REV	BY	DATE

MillerCoors

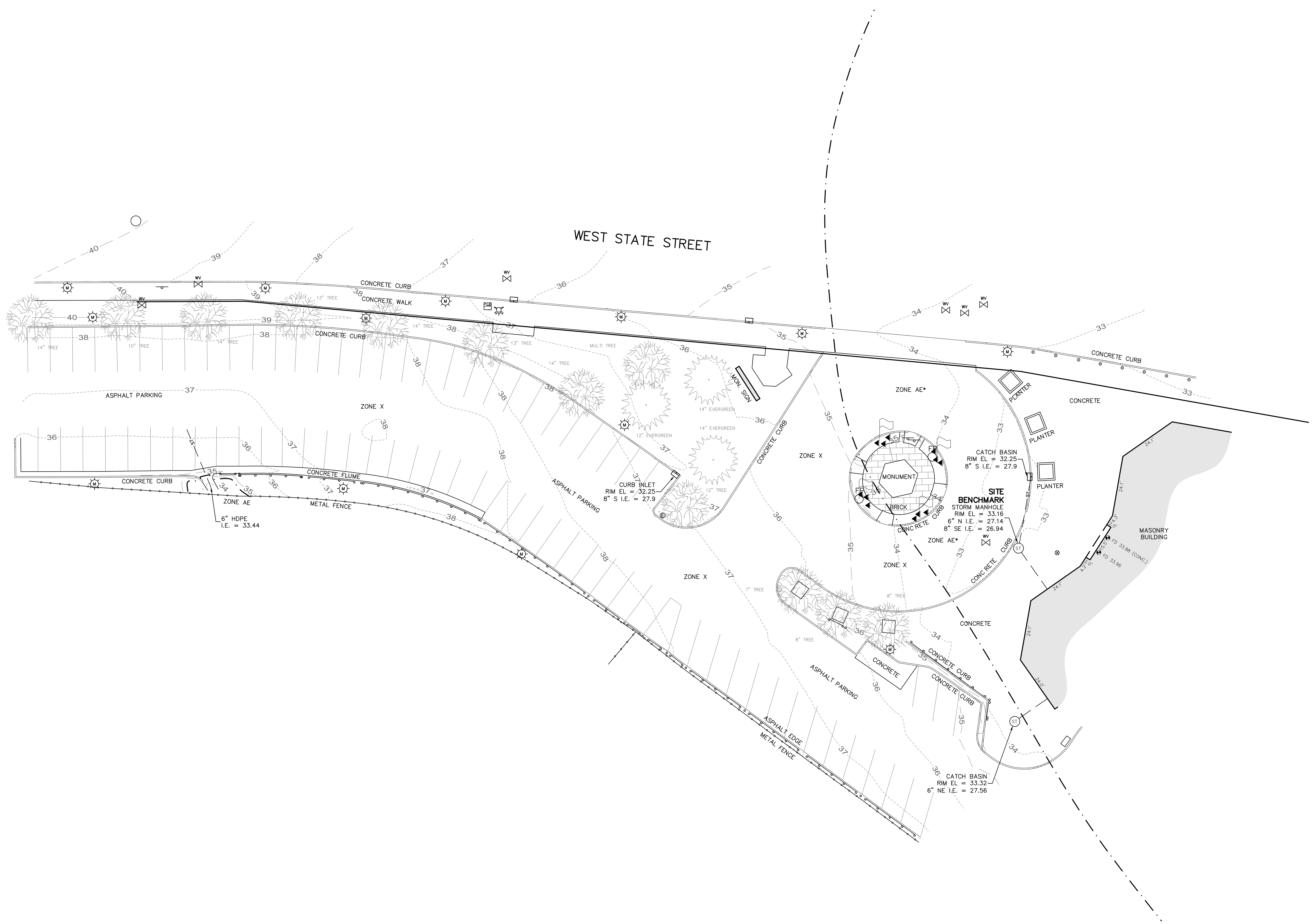
MILLERCOORS PROJECT NUMBER:	122612	
PROJECT LEAD DESIGNER:	ADLER	
PLANT:	MILWAUKEE	PROPOSED ARCHITECTURAL SITE PLAN
DATE:	01/09/18	HISTORIC PRESERVATION SUBMITTAL
JAK PROJECT NUMBER:	17047-00	
JAK PROJECT MANAGER:	DK	

	INITIAL	DATE
DR.		
CH.		
APPR.		
SCALE		

SUBJECT BLDG. NO. RELEASE NO. SIZE

AS101

CSE
CAPITOL SURVEY ENTERPRISES
220 REGENCY CT. STE. 210
BROOKFIELD, WI 53045
Ph: (262) 786-6600
Fax: (414) 786-6608
www.capitolsurvey.com



NOTES

1. THE UNDERGROUND UTILITY INFORMATION AS SHOWN HEREON IS BASED, IN PART, ON INFORMATION FURNISHED BY THE UTILITY COMPANIES, DIGGERS HOTLINE AND THE LOCAL MUNICIPALITY. WHILE THIS INFORMATION IS BELIEVED TO BE RELIABLE, ITS ACCURACY AND COMPLETENESS CANNOT BE GUARANTEED NOR CERTIFIED TO.

2. SUBJECT PROPERTY ARE LOCATED WITHIN AN AREA HAVING A ZONE DESIGNATION X: AREAS DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOOD PLAIN, AE: SPECIAL FLOOD HAZARD AREAS (SFHAs) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD WITH BASE FLOOD ELEVATIONS DETERMINED PER INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA), ON FLOOD INSURANCE RATE MAP NO. 55079C0087E, WITH A DATE OF IDENTIFICATION OF SEPTEMBER 26, 2008, IN COMMUNITY NO. 550278, THE CITY OF MILWAUKEE, WHICH IS THE COMMUNITY IN WHICH THE SUBJECT PROPERTY IS SITUATED.

4. AREAS MARKED "ZONE AE*" DETERMINED BY GRAPHICAL DEPICTION FROM FIRM MAP ONLY. FURTHER INQUIRY REQUIRED FOR DETERMINED ELEVATIONS MARKING THE LIMITS OF FLOOD ZONE SHOWN, IF FLOOD ZONE SHOWN CAN BE DETERMINED BY ELEVATION.

5. PROJECT BENCHMARK – CONCRETE MONUMENT WITH ALUMINUM CAP FOUND IN TOP OF BRIDGE PARAPET WALL 3.6' ABOVE CONCRETE WALK ON NORTH SIDE OF W. WISCONSIN AVE, 139.97 FEET NORTHWESTERLY OF THE EAST CORNER OF SECTION 26–7–21, EL = 88.73.

6. SITE BENCHMARK – NORTH RIM OF STORM MANHOLE RIM, AS SHOWN HEREON, EL = 33.16.

7. ELEVATIONS BASED ON INFORMATION FROM THE SEWRPC OF AND ARE AT CITY OF MILWAUKEE DATUM.

LEGEND

— SAN —	SANITARY SEWER	☒	ELECTRIC TRANSFORMER		HYDRANT
— ST —	STORM SEWER	□EM	ELECTRIC METER	WV	WATER VALVE
— W —	WATER MAIN	□EP	ELECTRIC PEDESTAL	GV	GAS VALVE
— G —	BURIED GAS LINE	□EB	ELECTRIC BOX AT GRADE	MH	MANHOLE
— TEL —	BURIED TELEPHONE LINE	□TB	TELEPHONE BOX AT GRADE	ST	STORM MANHOLE
— E —	BURIED ELECTRIC LINE	□TP	TELEPHONE PEDESTAL	CB	CATCH BASIN
— FO —	BURIED FIBER OPTIC LINE	□TV	TV PEDESTAL		CURB INLET
— // —	OVERHEAD UTILITY LINES	□GM	GAS METER	M	METAL LIGHT POLE
— CATV —	BURIED CABLE TELEVISION LINES	A	AIR CONDITIONER	C	CONCRETE LIGHT POLE
— COMB —	COMBINATION SEWER		UTILITY POLE	W	WOOD LIGHT POLE
	WOOD FENCE		WOOD SIGN	MB	MAIL BOX
	METAL FENCE		METAL SIGN	F	FIBER OPTIC MARKER
	EDGE OF TREES AND BRUSH		FLAG POLE	GUY	GUY WIRE
33.88 DS	DOOR SILL ELEVATION		BOLLARD		
	FIRE DEPARTMENT CONNECTION		BOLLARD LIGHT		
			YARD LIGHT		

EXISTING CONDITIONS SURVEY

FOR

HISTORIC MC GETTELMAN BUILDING

4315 W. STATE ST.
MILWAUKEE, WI

Drawn By:	NJF	Date:	NOV. 20, 2017		
Checked By:	MJB	Drawing No.	EC-093		
CSE Job No.:	17-093	Sheet	1	of	1

Graphic Scale
20 0 10 20 40
(IN FEET)
1 inch = 20 ft.

NOTE: SURVEY COMPLETED BY CAPITOL SURVEY ENTERPRISES. THE ENGINEER MAKES NO WARRANTY OR REPRESENTATION WITH REFERENCE TO THE ACCURACY AND COMPLETENESS OF THE EXISTING CONDITIONS INDICATED OR NOT INDICATED ON THE ENGINEERING PLANS PROVIDED.

Harwood Engineering Consultants
255 North 21st Street Milwaukee, WI 53233

MillerCoors
MILWAUKEE, WISCONSIN

MILLERCOORS PROJECT NUMBER: 122612
PROJECT DESIGN LEAD: ADLER

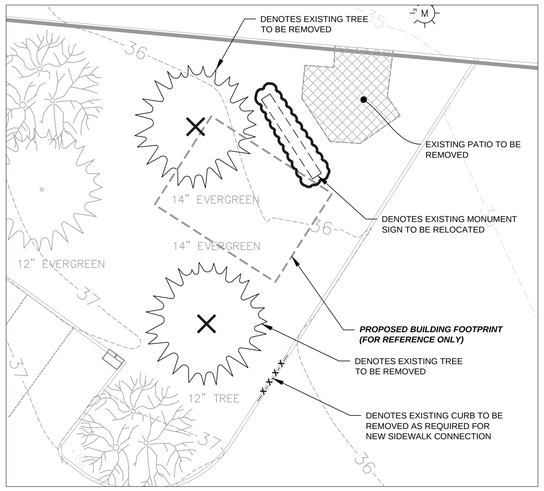
PLANT: MILWAUKEE	EXISTING SURVEY
DATE: 01/09/18	HISTORIC PRESERVATION SUBMITTAL

JAK architecture + interior design
JAKnetter Architects
N16 W23217 STONE RIDGE DRIVE, SUITE 300 WAUKESHA, WI 53188
www.jaknetter.com | office 262 513 9800 | fax 262 513 9815

JAK PROJECT NUMBER: 17047-00
JAK PROJECT MANAGER: DK

DESCRIPTION	REV.	CHANGED BY	DATE

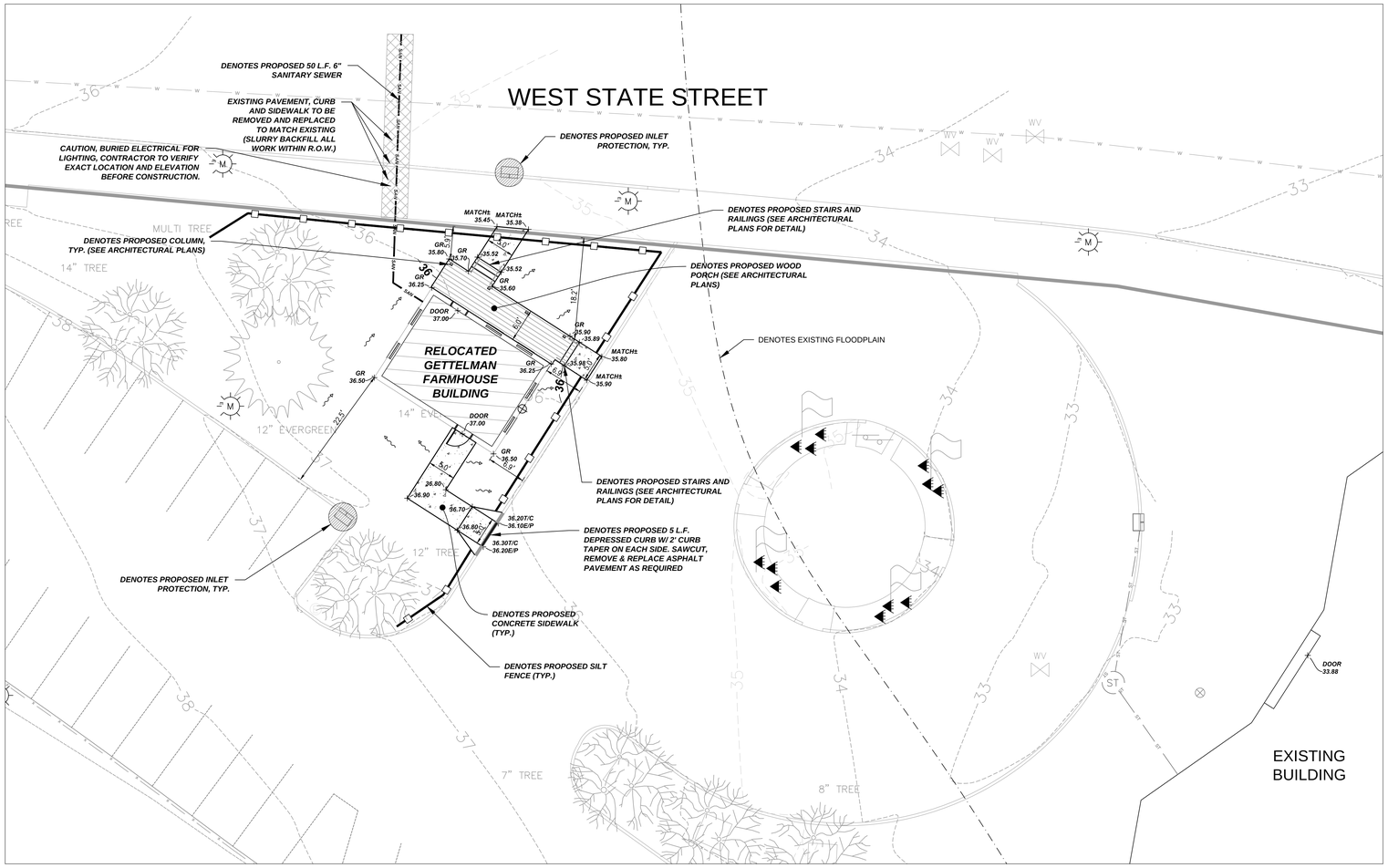
	INITIAL	DATE	SUBJECT BLDG. NO. RELEASE NO. SIZE
DR.			C1.20
CH.			
APPR.			
SCALE			



DEMOLITION PLAN

SCALE: 1" = 10'

North



SITE GRADING & EROSION CONTROL PLAN

SCALE: 1" = 10'

North

EROSION CONTROL NOTES AND PHASING

ESTIMATED CONSTRUCTION TIMEFRAMES:
INSTALL EROSION CONTROL = MAY, 2018
GRADING AND UTILITY INSTALLATION = MAY, 2018
FINAL SITE GRADING AND RESTORATION = JULY, 2018

ALL CHANGES TO THE ABOVE SCHEDULE SHALL BE REVIEWED AND APPROVED BY THE MUNICIPALITY.

1. CONTRACTOR SHALL INSPECT ALL EROSION CONTROL PRACTICES WEEKLY AND AFTER ANY RAINFALL EVENT OF 0.5 INCHES OR GREATER. THE CONTRACTOR SHALL PERFORM ALL INSPECTIONS AND DOCUMENTATION PER THE WISCONSIN DEPARTMENT OF NATURAL RESOURCES. ALL REQUIRED REPAIRS SHALL BE MADE WITHIN 24 HOURS.
2. PRIOR TO BEGINNING CONSTRUCTION, THE CONTRACTOR WILL HAVE IN PLACE, ALL APPLICABLE PLAN APPROVALS AND PERMITS.
3. INSTALL INLET PROTECTION WHERE INDICATED ON PLANS.
4. STRIP TOPSOIL FROM THE SITE (WHERE PROPOSED IMPROVEMENTS OR GRADING IS SHOWN ONLY). TOPSOIL STOCKPILE(S) REMAINING FOR MORE THAN SEVEN DAYS SHALL BE STABILIZED WITH VEGETATIVE COVER, MULCH, TARPS OR OTHER APPROVED PRACTICE. EROSION FROM TOPSOIL PILES LEFT FOR LESS THAN SEVEN DAYS SHALL BE CONTROLLED WITH SILT FENCE OR OTHER APPROVED METHOD. ANY TOPSOIL STOCKPILE WITHIN 25' OF A ROADWAY OR DRAINAGE DITCH SHALL BE COVERED WITH TARPS OR OTHER APPROVED METHOD. ALL DISTURBED GROUND LEFT INACTIVE FOR SEVEN OR MORE DAYS IS TO BE STABILIZED BY SEED, SOD, MULCH, OR OTHER APPROVED METHOD.
5. INSTALL UTILITIES
6. REDISTRIBUTE TOPSOIL FROM STOCKPILE(S) TO A DEPTH OF 6 INCHES. SURPLUS TOPSOIL SHALL BE REMOVED FROM THE SITE BY THE CONTRACTOR, COORDINATE W/ OWNER. FINAL GRADE, SEED AND MULCH SITE. PLACE EROSION CONTROL MATTING WHERE INDICATED ON PLANS. (SEEDING AND MULCHING TO CONFORM WITH APPROVED SEED MIXTURES AND APPLICATION RATES, SEE LANDSCAPE PLAN FOR FINAL SEED AND SOD SPECS. EROSION CONTROL MATTING TO BE INSTALLED PER MANUFACTURE'S SPECIFICATIONS.)
7. INSTALL AGGREGATE BASE COURSE IN AREAS TO BE CONCRETE PAVED
8. INSTALL CONCRETE SECTIONS.
9. UPON SITE STABILIZATION, REMOVE TEMPORARY EROSION CONTROL PRACTICES. CLEAN STRUCTURES OF ANY SEDIMENT AND/OR CONSTRUCTION DEBRIS.
10. CONSTRUCTION AND WASTE MATERIALS SHALL BE PROPERLY DISPOSED OF ON A ROUTINE BASIS. NO CONSTRUCTION OR WASTE MATERIALS SHALL BE TRACKED, BLOWN OR OTHERWISE LOCATED OR STORED ON ADJACENT PROPERTIES.
11. DUST CONTROL SHALL BE MAINTAINED ONSITE WITH USE OF A WATER TRUCK (IF NECESSARY).

DEMOLITION LEGEND

- DENOTES ITEM TO BE REMOVED OR RELOCATED
- DENOTES CONCRETE REMOVAL AREA
- DENOTES TREE TO BE REMOVED
- SILT FILTER FENCE

HATCH LEGEND

- PROPOSED CONCRETE SIDEWALK

UTILITY LEGEND

SYMBOL	DESCRIPTION
W W	EXISTING WATER MAIN
W W	PROPOSED WATER SERVICE
E E	EXISTING ELECTRICAL LINE
E E	PROPOSED ELECTRICAL LINE
GAS GAS	EXISTING GAS MAIN
GAS GAS	PROPOSED GAS MAIN
SAN SAN SAN	EXISTING SANITARY SEWER
SAN SAN SAN	PROPOSED SANITARY SEWER
ST ST ST	EXISTING STORM SEWER
ST ST ST	PROPOSED STORM SEWER
OHW OHW	OVERHEAD WIRES
	EXISTING POWER POLES
W	EXISTING LIGHT POLES
S	SANITARY MANHOLE
	FIRE HYDRANT
WV	EXISTING WATER VALVE
	PROPOSED WATER VALVE
D	EXISTING STORM STRUCTURE
D	PROPOSED STORM STRUCTURE
	DENOTES EMERGENCY OVERFLOW ROUTE / DRAINAGE PATH
100.00 100.00	PROPOSED & EXISTING SPOT GRADE

DIGGER'S HOTLINE — CALL BEFORE YOU DIG — 1-800-242-8511

IN ACCORDANCE WITH WISCONSIN STATUTE 182.0175, DAMAGE TO TRANSMISSION FACILITIES, EXCAVATOR SHALL BE SOLELY RESPONSIBLE TO PROVIDE ADVANCE NOTICE TO THE DESIGNATED "ONE CALL SYSTEM" NOT LESS THAN THREE WORKING DAYS PRIOR TO COMMENCEMENT OF ANY EXCAVATION REQUIRED TO PERFORM WORK CONTAINED ON THESE DRAWINGS, AND FURTHER, EXCAVATOR SHALL COMPLY WITH ALL OTHER REQUIREMENTS OF THIS STATUTE RELATIVE TO EXCAVATOR'S WORK.

Harwood Engineering Consultants
255 North 21st Street | Milwaukee, WI 53233

DESCRIPTION	REV.	CHANGED BY	DATE

MillerCoors
MILWAUKEE, WISCONSIN

MILLERCOORS PROJECT NUMBER: 122612
PROJECT DESIGN LEAD: ADLER

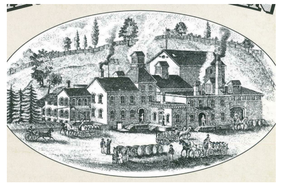
PLANT: MILWAUKEE | SITE GRADING EROSION CONTROL & DEMOLITION PLAN
DATE: 01/09/18 | HISTORIC PRESERVATION SUBMITTAL

JAK Architecture + Interior Design

JAK PROJECT NUMBER: 17047-00
JAK PROJECT MANAGER: DK

	INITIAL	DATE	SUBJECT BLDG. NO. RELEASE NO.SIZE
DR.			C1.10
CH.			
APPR.			
SCALE			

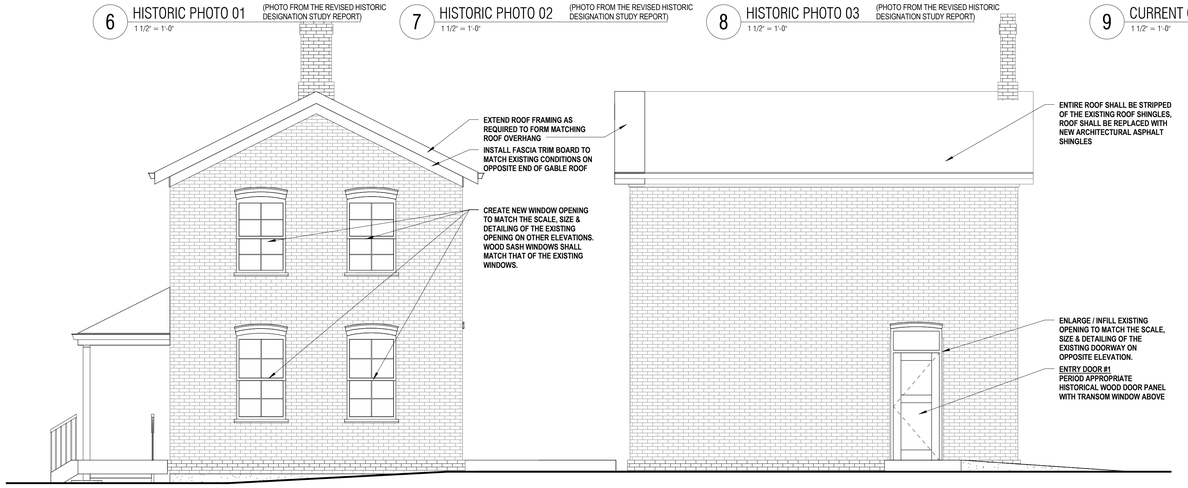
6 HISTORIC PHOTO 01
1 1/2" = 1'-0"
(PHOTO FROM THE REVISED HISTORIC DESIGNATION STUDY REPORT)



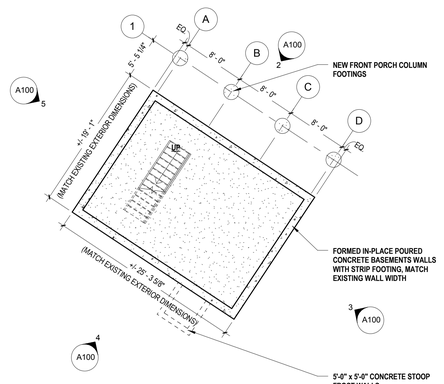
7 HISTORIC PHOTO 02
1 1/2" = 1'-0"
(PHOTO FROM THE REVISED HISTORIC DESIGNATION STUDY REPORT)

8 HISTORIC PHOTO 03
1 1/2" = 1'-0"
(PHOTO FROM THE REVISED HISTORIC DESIGNATION STUDY REPORT)

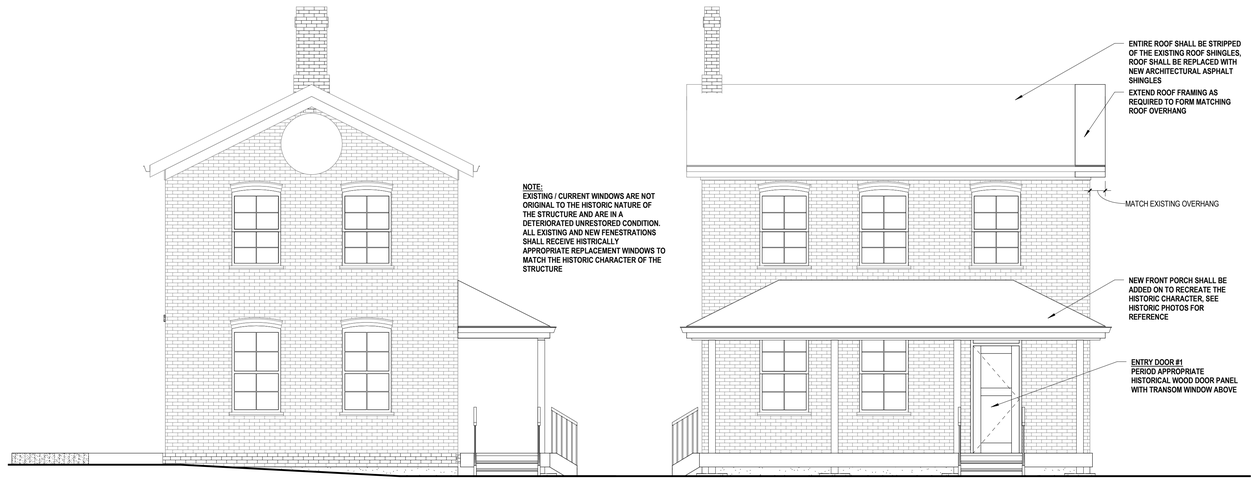
9 CURRENT CONDITIONS
1 1/2" = 1'-0"
(PHOTO FROM THE REVISED HISTORIC DESIGNATION STUDY REPORT)



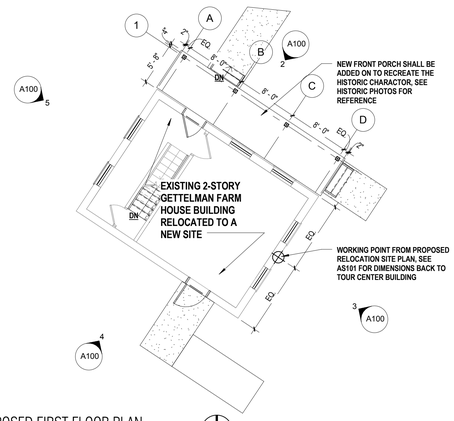
5 PROPOSED "WEST" ELEVATION
1/4" = 1'-0"
(FORMER NORTH ELEVATION)



10 PROPOSED BASEMENT FLOOR PLAN
1/8" = 1'-0"



3 PROPOSED "EAST" ELEVATION
1/4" = 1'-0"
(FORMER SOUTH ELEVATION)



2 PROPOSED "NORTH" ELEVATION
1/4" = 1'-0"
(FORMER EAST ELEVATION)

1 PROPOSED FIRST FLOOR PLAN
1/8" = 1'-0"

BUILDING ASSEMBLIES

FOUNDATION SYSTEM #1
1'-0" x 2'-0" TYPICAL CONCRETE FOOTING WITH FORMED POURED-IN-PLACE CONCRETE FOUNDATION WALLS. SEE STRUCTURAL DRAWINGS FOR SPECIFIC INFORMATION. PROVIDE 2" RIGID INSULATION VERTICALLY ON THE BACKSIDE OF THE FOUNDATION WALL FROM UNDERSIDE OF SLAB TO TOP OF FOOTING-4'-0" HIGH. (OCCURS AT ALL EXTERIOR WALL CONDITIONS).

FOUNDATION SYSTEM #2
MINIMUM 24" ROUND CONCRETE PEIR FOOTINGS. SEE STRUCTURAL DRAWINGS FOR SPECIFIC INFORMATION.

FLOOR #1 (BASEMENT)
4" CONCRETE SLAB W/ 6x6 W1.4xW1.4 WWF OVER 10-MIL POLYETHYLENE VAPOR BARRIER AND 6" FREELY DRAINING COMPACTED GRANULAR FILL SUB-BASE. PROVIDE PRE-MOLDED JOINT FILL AT PERIMETER SLAB JOINT CONDITIONS. (SEE STRUCTURAL DRAWINGS FOR REINFORCING, CONTROL JOINT LOCATIONS AND SPECIFIC INFORMATION). PROVIDE 2" RIGID INSULATION HORIZONTALLY 4'-0" BELOW SLAB FROM BACKSIDE OF FOUNDATION WALL AROUND BUILDING PERIMETER.

FLOOR #2 (1ST FLOOR)
2x WOOD FRAMING CENTERED BETWEEN EXISTING FLOOR JOIST TO REINFORCE EXISTING FLOOR STRUCTURE. SEE STRUCTURAL DRAWINGS FOR SPECIFIC INFORMATION.

EXTERIOR WALL SYSTEM #1
EXISTING MULTI-WYTHE LOAD BEARING BRICK CONSTRUCTION. REPAIR OR REPLACE DETERIORATED AND MISSING MASONRY WITH SALVAGED MATERIALS THAT DUPLICATES THE EXISTING.

EXTERIOR WALL RE-TUCKPOINTING #1
RE-TUCKPOINT DEFECTIVE MORTAR BY DULICATING THE EXISTING COLOR, HARDNESS, TEXTURE AND JOINT FINISH. CONSULTATION WITH HISTORIC PRESEVATION STAFF IS REQUIRED BEFORE ANY WORK MAY BEGIN.

EXTERIOR WALL CLEANING #1
EXISTING EXTERIOR BRICK SHALL BE CLEANED BY REMOVING ALL OF THE EXISTING VINES AND FURTHER CLEANING SHALL BE DONE WITH THE MOST GENTLE METHOD POSSIBLE. CHEMICAL CLEANING SHALL ONLY BE DONE BY EXPERIENCED CRAFTSMAN. CONSULTATION WITH HISTORIC PRESEVATION STAFF IS REQUIRED BEFORE ANY WORK MAY BEGIN.

FLASHING SYSTEM #1
AT NEW PORCH STRUCTURE #1 PROVIDE PRE-FINISHED, 22 GA METAL COUNTER FLASHING WITH METAL REGLET SAWCUT INTO MORTAR OR RETURN AND TERMINATE TO ALUMINUM SYSTEM. INSTALL SEALANT AT METAL FLASHING AND FACE BRICK JOINT.

WINDOW SYSTEM #1
VISION GLASS: 1" INSULATED CLEAR LOW-E GLASS. WOOD WINDOW: RETAIN EXISTING CONFIGURATION OF HOODS, SASHES, SURROUNDS AND SILLS; EXCEPT WHERE NECESSARY TO RESTORE THEM TO ORIGINAL CONDITION. ONLY PERIOD APPROPRIATE HISTORICAL WOOD DOUBLE-HUNG WINDOW REPLACEMENTS SHALL BE USED; MODERN VINYL, VINYL CLAD, METAL, METAL CLAD OR FIBERGLASS WINDOW UNITS ARE NOT PERMITTED. CONSULTATION WITH HISTORIC PRESEVATION STAFF IS REQUIRED BEFORE ANY WORK MAY BEGIN.

ENTRY DOOR #1
WOOD DOOR: RETAIN EXISTING CONFIGURATION OF HOODS, SASHES, SURROUINDS AND SILLS; EXCEPT WHERE NECESSARY TO RESTORE THEM TO ORIGINAL CONDITION. ONLY PERIOD APPROPRIATE HISTORICAL WOOD PANEL DOOR REPLACEMENTS SHALL BE USED; MODERN VINYL, VINYL CLAD, METAL, METAL CLAD OR FIBERGLASS DOOR UNITS ARE NOT PERMITTED. CONSULTATION WITH HISTORIC PRESEVATION STAFF IS REQUIRED BEFORE ANY WORK MAY BEGIN.

TRIM #1
EXISTING HISTORICAL TRIM AND/OR ORNAMENTATION SHALL REMAIN. SPOT REPAIR / REPLACEMENT OF ANY DETERIORATED MATERIAL AS NECESSARY VERSES COMPLETE REMOVAL AND REPLACEMENT. ANY REPLACEMENT MATERIALS SHALL MATCH THE ORIGINAL MATERIALS IN TERMS OF SCALE, DESIGN, COLOR AND WOOD SPECIES. CONSULTATION WITH HISTORIC PRESEVATION STAFF IS REQUIRED BEFORE ANY WORK MAY BEGIN.

PORCH STRUCTURE #1
PRIME AND EXTERIOR PAINT EXPOSED STRUCTURAL WOOD SUPPORTS AND FRAMING. SKELETON FRAME ATTACHED TO THE BUILDING FAÇADE WITH HIDDEN CONNECTIONS TO THE EXISTING EXTERIOR WALLS. FRAME UNDERSIDE OF PORCH WITH CAR SIDING AND BEAD BOARD SOFFIT MATERIAL TO BE PAINTED. PROVIDE AND INSTALL ARCHITECTURAL ASPHALT SHINGLES AND ALUMNUM GUTTERS. COLOR TBD. CONSULTATION WITH HISTORIC PRESEVATION STAFF IS REQUIRED BEFORE ANY WORK MAY BEGIN.

ROOF SYSTEM #1
PROVIDE AND INSTALL ARCHITECTURAL ASPHALT SHINGLES AND MINIMUM OF 3'-0" WIDE ICE-N-WATER SHEILD AT ALL EAVES. COLOR TBD. CONSULTATION WITH HISTORIC PRESEVATION STAFF IS REQUIRED BEFORE ANY WORK MAY BEGIN.

SITE REFERENCE PLAN
NTS

DESCRIPTION	REV	BY	DATE

MillerCoors
MILLERCOORS PROJECT NUMBER: 122612
PROJECT LEAD DESIGNER: ADLER
PLANT: MILWAUKEE | GROUND LEVEL FLOOR PLAN
DATE: 01/09/18 | HISTORIC PRESERVATION SUBMITTAL
JAK architecture interior design
JAK PROJECT NUMBER: 17047-00
JAK PROJECT MANAGER: DK

	INITIAL	DATE
DR.		
CH.		
APPR.		
SCALE		

SUBJECT BLDG. NO. RELEASE NO. SIZE
A100

GENERAL NOTES AND SPECIFICATIONS

1. THE EXISTING SITE INFORMATION ON THIS PLAN WAS TAKEN FROM A SITE SURVEY PROVIDED BY CAPITOL SURVEY ENTERPRISES. THE ENGINEER MAKES NO WARRANTY OR REPRESENTATION WITH REFERENCE TO THE ACCURACY AND COMPLETENESS OF THE EXISTING CONDITIONS INDICATED OR NOT INDICATED ON THE ENGINEERING PLANS PROVIDED. THE CONTRACTOR SHALL VERIFY THE LOCATION OF ALL EXISTING SITE CONDITIONS INCLUDING UNDERGROUND UTILITIES, UNDERGROUND UTILITY ELEVATIONS, BUILDING SETBACKS AND EXISTING BUILDING LOCATIONS. THE CONTRACTOR SHALL INFORM THE OWNER AND ENGINEER OF ANY DISCREPANCIES PRIOR TO COMMENCING WITH WORK. QUESTIONS REGARDING THE EXISTING SURVEY SHALL BE DIRECTED TO THE PARTIES LISTED ABOVE.
2. BEFORE PROCEEDING WITH ANY UTILITY CONSTRUCTION, CONTRACTOR SHALL EXCAVATE EACH EXISTING LATERAL TO BE CONNECTED TO (VERIFYING ELEVATION, LOCATION AND SIZE). SHOULD THE EXISTING UTILITY NOT BE AS INDICATED ON THE PLAN, THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY FOR EVALUATION.
3. ALL UTILITY CONSTRUCTION SHALL ADHERE TO THE STANDARD SPECIFICATIONS FOR SEWER AND WATER CONSTRUCTION IN WISCONSIN (2003), AS WELL AS, THE CITY OF WEST ALLIS CONSTRUCTION STANDARDS AND THE DEPT. OF SAFETY AND PROFESSIONAL SERVICED SEC. 382-387.
4. ALL UTILITY PERMITS MUST BE RECEIVED FROM THE CITY OF WEST ALLIS PRIOR TO THE START OF CONSTRUCTION.
5. NOTIFY THE PUBLIC WORKS INSPECTION DEPT. AT LEAST 48 HOURS BEFORE STARTING CONSTRUCTION.
6. BACKFILL REQUIREMENTS AND ROADWAY/SIDEWALK RESTORATION SHALL ADHERE TO LOCAL STANDARDS (GRANULAR BACKFILL UNDER OR WITHIN 5' OF CURBS, SIDEWALK, OR PAVEMENT. SPOIL MAY BE USED ELSEWHERE. SLURRY BACKFILL WILL BE REQUIRED IN PUBLIC ROADWAYS.)
7. ALL BUILDING UTILITIES SHALL BE VERIFIED WITH THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION.
8. PROPOSED STORM SEWER SHALL BE PVC, ASTM D-3034, SDR 35 WITH RUBBER ELASTOMERIC JOINTS CONFORMING TO ASTM D-3212 (UNLESS OTHERWISE NOTED).
9. UTILITY TRENCHES SHALL BE MECHANICALLY COMPACTED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR SEWER AND WATER CONSTRUCTION IN WISCONSIN.
10. ALL EROSION CONTROL METHODS MUST BE INSTALLED PRIOR TO COMMENCEMENT OF CONSTRUCTION. ALSO, CONTRACTOR IS RESPONSIBLE FOR REMOVING EROSION CONTROL METHODS ONCE THE SITE IS STABILIZED.
11. THE PROPOSED SITE LOCATION AND SURROUNDING STREETS MUST BE KEPT DEBRIS FREE. SWEEP STREETS AS NEEDED TO MAINTAIN CLEAN STREETS.
12. ALL EXCAVATED OR STRIPPED MATERIALS NOT BEING REPLACED IN UTILITY TRENCHES OR BEING USED FOR FILL SHALL BE REMOVED FROM THE SITE, UNLESS OTHERWISE DIRECTED BY THE OWNER.
13. ALL DISTURBED GRASS AREAS SHALL BE STABILIZED (PER DNR TECHNICAL STANDARDS) WITHIN 7 DAYS OF COMPLETION. DISTURBED GRASS AREAS SHALL BE TOPSOILED (6"), RESEEDED AND STABILIZED. AREAS WITH A SLOPE OF 3H:1V OR STEEPER SHALL BE COVERED WITH A CLASS 1 - TYPE A EROSION FABRIC. (SEE SPECIFICATIONS)
14. SEE ARCHITECTURAL PLANS FOR EXACT BUILDING & FOUNDATION DETAILS AND ORIENTATION.
15. CONTRACTOR SHALL MATCH PROPOSED CONCRETE AND ASPHALT PAVEMENT TO EXISTING IN ELEVATION AND ALIGNMENT.
16. REMOVAL OF PAVEMENT SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS OF THE WISCONSIN D.O.T.
17. ALL CONCRETE MUST CONFORM TO THE STANDARD SPECIFICATIONS FOR READY MIXED CONCRETE. MINIMUM 28 DAY COMPRESSIVE STRENGTH TEST MUST EQUAL 4000 PSI.
18. CONTRACTOR IS RESPONSIBLE FOR PROTECTING ALL PROPERTY CORNERS.
19. CONTRACTOR IS RESPONSIBLE FOR REPAIRING ANY DAMAGE TO EXISTING UTILITIES OR SITE IMPROVEMENTS. CONTRACTOR SHALL DOCUMENT ALL EXISTING DAMAGE PRIOR TO START OF CONSTRUCTION AND NOTIFY CONSTRUCTION MANAGER OF ANY FINDINGS.
20. PROJECT SAFETY ON-SITE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
21. CONTRACTOR IS RESPONSIBLE FOR VERIFYING EXISTING SOIL CONDITIONS, CONSTRUCTION MANAGER MAY HAVE SOILS REPORT FOR MORE INFO.
22. CONTRACTOR IS RESPONSIBLE FOR PROVIDING THE OWNER WITH A SET OF MARKED UP PLANS (AS-BUILTS) SHOWING ANY CHANGES DURING CONSTRUCTION.

DENSE GRADED BASE

1. MATERIALS SHALL CONFORM TO SECTION 301.2 OF THE WISDOT STANDARD SPECIFICATIONS FOR HIGHWAY AND STRUCTURE CONSTRUCTION. MATERIAL GRADATIONS SHALL CONFORM TO SECTION 305.2.2 OF THE WISDOT STANDARD SPECIFICATIONS FOR HIGHWAY AND STRUCTURE CONSTRUCTION UNLESS SPECIFIED ELSEWHERE IN THE CONTRACT DOCUMENTS.
2. BASE COURSE MATERIAL SHALL BE CRUSHED STONE OR CRUSHED GRAVEL ONLY.
3. PREPARE THE FOUNDATION, OR RESURFACE THE PREVIOUSLY PLACED BASE LAYER, AS SPECIFIED IN WISDOT SECTION 211 BEFORE PLACING BASE. DO NOT PLACE BASE FOUNDATIONS THAT ARE SOFT, SPONGY, OR COVERED BY ICE OR SNOW. WATER AND REWORK OR RE-COMPACT DRY FOUNDATIONS AS NECESSARY TO ENSURE PROPER COMPACTION, OR AS THE REPRESENTATIVE DESIGNATES.
4. IN PROPOSED PAVEMENT AREAS, ALL ORGANIC SOLID SHALL BE REMOVED.
5. IN AREAS OF EXISTING PAVEMENT TO BE MODIFIED OR ADJUSTED IN GRADE, THE EXISTING PAVEMENT SECTION SHALL BE REMOVED BY AN ACCEPTABLE METHOD. THE NEW PAVEMENT SECTION SHALL MATCH THE CONSTRUCTION DETAILS.
6. PROOF-ROLL ALL SUBGRADE AREAS THAT ARE TO RECEIVE AGGREGATE BASE OR PAVEMENT.
7. BUILD AND MAINTAIN STOCKPILES USING METHODS THAT MINIMIZE SEGREGATION AND PREVENT CONTAMINATION. IF THE CONTRACT SPECIFIES LOCATION, PLACE STOCKPILES WHERE SPECIFIED. CLEAR AND PREPARE STOCKPILE AREAS TO FACILITATE THE RECOVERY OF THE MAXIMUM AMOUNT OF STOCKPILED MATERIAL.
8. PLACE AGGREGATE IN A MANNER THAT MINIMIZES HAULING ON THE SUBGRADE. DO NOT USE VEHICLES OR OPERATIONS THAT DAMAGE THE SUBGRADE OR IN-PLACE BASE. DEPOSIT MATERIAL IN A MANNER THAT MINIMIZES SEGREGATION.
9. COMPACT THE BASE UNTIL THERE IS NO APPRECIABLE DISPLACEMENT, EITHER LATERALLY OR LONGITUDINALLY, UNDER THE COMPACTION EQUIPMENT.
10. COMPACT EACH BASE LAYER, INCLUDING SHOULDER FORESLOPES, WITH EQUIPMENT SPECIFIED IN WISDOT SECTION 301.3.1. USE STANDARD COMPACTION CONFORMING TO WISDOT SECTION 301.3.4.2, UNLESS THE SPECIAL PROVISIONS SPECIFY OTHER METHODS. FINAL SHAPING OF SHOULDER FORESLOPES DOES NOT REQUIRE COMPACTION.
11. AFTER THE PROJECT IS COMPLETED, THOROUGHLY CLEAN UP ALL DEBRIS WHICH MAY HAVE ACCUMULATED DURING THE PLACEMENT OF DENSE GRADED BASE, REPLACE OR REPAIR AS REQUIRED, ALL SURFACES AND/OR LANDSCAPE FEATURES DAMAGED OR DISTURBED UNDER THIS ITEM OF WORK.

CAST IN PLACE CONCRETE

1. ALL WORK SHALL BE IN ACCORDANCE WITH APPLICABLE MANUFACTURER'S AND SUPPLIER'S INSTRUCTIONS.
2. ALL CONCRETE WORK WHICH DOES NOT CONFORM TO THE REQUIREMENTS OF THE CONTRACT DOCUMENTS AND ACI 301, INCLUDING FUNCTION, DURABILITY, APPEARANCE, STRENGTH, CRACKING, TOLERANCES AND FINISHING, SHALL BE CORRECTED AS DIRECTED BY ARCHITECT AT CONTRACTOR'S EXPENSE. ADDITIONAL TESTING, ENGINEERING, REINFORCEMENT AND REMOVAL AND REPLACEMENT OF DEFECTIVE CONCRETE SHALL BE PAID FOR BY CONCRETE CONTRACTOR. CONTRACTOR SHALL ALSO BE RESPONSIBLE FOR THE COST OF CORRECTIONS TO ANY OTHER WORK AFFECTED BY OR RESULTING FROM CORRECTIONS TO THE CONCRETE WORK.
3. CONCRETE SHALL CONFORM TO SECTIONS 501 AND 601 OF THE STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION.
4. ALL CONCRETE, UNLESS OTHERWISE SPECIFICALLY PERMITTED BY ARCHITECT, SHALL BE TRANSIT-MIXED IN ACCORDANCE WITH ASTM C 94.
5. IN GENERAL, COMPLY WITH ASTM C 33 FOR GRADING AND QUALITY OF FINE AND COARSE AGGREGATE FOR USE IN CONCRETE.
6. PORTLAND CEMENT SHALL CONFORM WITH ASTM C 150 AND SHALL ONLY CONTAIN THE FOLLOWING INGREDIENTS: PORTLAND CEMENT CLINKER; WATER OR CALCIUM SULFATE, OR BOTH; LIMESTONE; PROCESSING ADDITIONS; AND AIR-ENTRAINING ADDITION FOR AIR-ENTRAINING PORTLAND CEMENT.
7. ADMIXTURES SHALL NOT CONTAIN MORE CHLORIDE IONS THAN ARE PRESENT IN MUNICIPAL DRINKING WATER.
8. WATER REDUCING ADMIXTURES SHALL CONFORM TO ASTM C 494.
9. AIR ENTRAINING ADMIXTURE SHALL CONFORM TO ASTM C 260
10. CALCIUM CHLORIDE, THIOCYANATES OR ADMIXTURES CONTAINING MORE THAN 0.05% CHLORIDE IONS BY WEIGHT OF ADMIXTURE ARE NOT PERMITTED FOR USE IN CONCRETE MIXES.
11. SYNTHETIC FIBERS SHALL BE USED IN CONCRETE MIX DESIGN IN LIEU OF WELDED WIRE FABRIC. SYNTHETIC FIBERS SHALL NOT REPLACE REINFORCING REBAR/DOWELS AS DEPICTED ON THE CONSTRUCTION DETAILS.
12. FOR CONCRETE PAVEMENTS: MATRIX HPS 950 MACRO/MICRO SYNTHETIC BLEND FIBER OR FORTA FERRO MACRO FIBER - FRC INDUSTRIES. APPLICATION DOSAGE SHALL BE 5 POUNDS PER CUBIC YARD.
13. CONCRETE MUST MEET ALL REQUIREMENTS OF THE ASTM C 94, ACI 211, ACI 318 CHAPTER 4 DURABILITY REQUIREMENTS, AND THOSE HEREIN SPECIFIED FOR MATERIALS, PROPORTIONING, MIXING AND OTHER DETAILS OF MANUFACTURER, QUALITY AND DELIVER.
14. AIR ENTRAINED CONCRETE: USE FOR ALL EXTERIOR SLABS, WALLS, WALKS, PLATFORMS, RAMPS, STEPS, ALL PORTIONS OF PARKING
15. MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS: 4000 PSI.
16. MAXIMUM AGGREGATE SIZE SHALL NOT EXCEED ONE THIRD OF THE SLAB ON GRADE THICKNESS.
17. FLY ASH MAY BE USED AS A POUND FOR POUND REPLACEMENT OF CEMENT UP TO 20% OF THE TOTAL CEMENTITIOUS CONTENT, 25% FOR FOOTINGS, EXCEPT FOR FINISHED FLATWORK DURING WINTER CONSTRUCTION, SUBJECT TO ARCHITECT'S APPROVAL.
18. CONCRETE REQUIRING AIR ENTRAINMENT SHALL CONTAIN SIX (6) PERCENT PLUS OR MINUS ONE AND A HALF (1.5) PERCENT AIR BY VOLUME, FOR 3/4" DIA. AGGREGATE. CONFORM TO ACI 318, CHAPTER 4.
19. ALL CONCRETE MUST CONTAIN THE SPECIFIED WATER-REDUCING ADMIXTURE OR WATER-REDUCING -RETARDING ADMIXTURE AND/OR THE SPECIFIED HIGH-RANGE WATER-REDUCING ADMIXTURE (SUPERPLASTICIZER). SPECIFIED CEMENT CONTENTS SHALL BE INCREASED 10 PERCENT (10%) WHEN NO WATER-REDUCING ADMIXTURES ARE USED.
20. MEASURING MATERIALS: CEMENT, AGGREGATES, WATER AND ADMIXTURES SHALL BE MEASURED AND COMBINED STRICTLY IN ACCORDANCE WITH ASTM SPECIFICATION C 94.
21. MAKE ONE SLUMP TEST OF THE FIRST TRUCK OF EACH MIX, EACH DAY, ONE TEST FOR EACH COMPRESSION TEST AND OTHER TESTS AS OFTEN AS REQUIRED THEREAFTER, WHENEVER CONSISTENCY CHANGES.
22. AIR CONTENT TESTS SHALL BE MADE FROM THE FIRST TRUCK OF EACH MIX, EACH DAY AND WHEN-EVER TEST CYLINDERS ARE MADE, IN ACCORDANCE WITH ASTM C 173 OR ASTM C231. TEST MORE OFTEN WHEN REQUIRED AIR CONTENTS ARE NOT ACHIEVED.
23. CONCRETE TEMPERATURE: TEST HOURLY WHEN AIR TEMPERATURE IS 40 DEGREES F (4 DEGREES C) AND BELOW, AND WHEN 80 DEGREES F (27 DEGREES C) AND ABOVE; AND EACH TIME A SET OF COMPRESSION TEST SPECIMENS IS MADE.
24. IF MEASURED SLUMP, AIR CONTENT OR CONCRETE TEMPERATURE FALLS OUTSIDE LIMITS SPECIFIED, A CHECK TEST SHALL BE MADE IMMEDIATELY ON ANOTHER PORTION OF SAME SAMPLE. IN EVENT OF A SECOND FAILURE, CONCRETE SHALL BE CONSIDERED TO HAVE FAILED TO MEET REQUIREMENTS OF SPECIFICATIONS AND SHALL NOT BE USED IN STRUCTURE. NOTIFY ARCHITECT IMMEDIATELY.
25. STRENGTH TESTS SHALL BE MADE FOR EACH OF THE FOLLOWING CONDITIONS: EACH DAY'S POUR, EACH CLASS OF CONCRETE, EACH CHANGE OF SUPPLIES OR SOURCE, EACH 150 CUBIC YARDS OF CONCRETE OR FRACTION THEREOF, AND EACH 5000 SQUARE FEET OF SURFACE AREA FOR SLABS OR WALLS.
26. TO CONFORM TO REQUIREMENTS OF THIS SPECIFICATION, THE STRENGTH LEVEL SHALL BE CONSIDERED SATISFACTORY SO LONG AS THE AVERAGE OF ALL SETS OF THREE (3) CONSECUTIVE STRENGTH TEST RESULTS EQUALS OR EXCEEDS THE SPECIFIED F'C AND NO INDIVIDUAL STRENGTH TEST RESULT FALLS BELOW THE SPECIFIED STRENGTH F'C BY MORE THAN 500 PSI. ARCHITECT SHALL BE NOTIFIED IMMEDIATELY OF NONCONFORMANCE.
27. BEFORE PLACING CONCRETE, VERIFY THAT INSTALLATION OF FORMWORK, REINFORCEMENT, AND EMBEDDED ITEMS IS COMPLETE AND THAT REQUIRED INSPECTIONS HAVE BEEN PERFORMED.
28. PROTECT FRESHLY PLACED CONCRETE FROM PREMATURE DRYING AND EXCESSIVE COLD OR HOT TEMPERATURES IN CONFORMANCE WITH ACI 301 AND ACI 308.
29. PROVIDE CONCRETE PAVEMENT HAVING THE THICKNESS AND REINFORCEMENT AS SHOWN ON THE DRAWINGS, OR TO MATCH ADJACENT EXISTING PAVEMENT. TIE BARS SHOULD BE PLACED AT ALL CONSTRUCTION JOINTS PARALLEL TO TRAFFIC AND CONSIST OF NO. 4 REINFORCING BARS, 24 INCHES IN LENGTH AND 48 INCHES ON CENTER, UNLESS OTHERWISE NOTED ON THE STANDARD DETAILS.

GENERAL NOTES:

DETAIL OF CONSTRUCTION NOT SHOWN ON THIS DRAWING SHALL CONFORM TO THE PERTINENT REQUIREMENTS OF THE STANDARD SPECIFICATIONS AND APPLICABLE SPECIAL PROVISIONS.

WHEN POSSIBLE THE SILT FENCE SHOULD BE CONSTRUCTED IN AN ARC OR HORSESHOE SHAPE, WITH THE ENDS POINTING UPSLOPE TO MAXIMIZE BOTH STRENGTH AND EFFECTIVENESS.

① CROSS BRACE WITH 2" x 4" WOODEN FRAME OR EQUIVALENT AT TOP OF POSTS AS DIRECTED BY THE ENGINEER.

② MINIMUM 14 GAGE WIRE REQUIRED, FOLD FABRIC 3" OVER THE WIRE AND STAPLE OR PLACE WIRE RINGS ON 12" C-C.

③ EXCAVATE A TRENCH A MINIMUM OF 4" WIDE AND 6" DEEP TO BURY AND ANCHOR THE GEOTEXTILE FABRIC, FOLD MATERIAL TO FIT TRENCH AND BACKFILL AND COMPACT TRENCH WITH EXCAVATED SOIL.

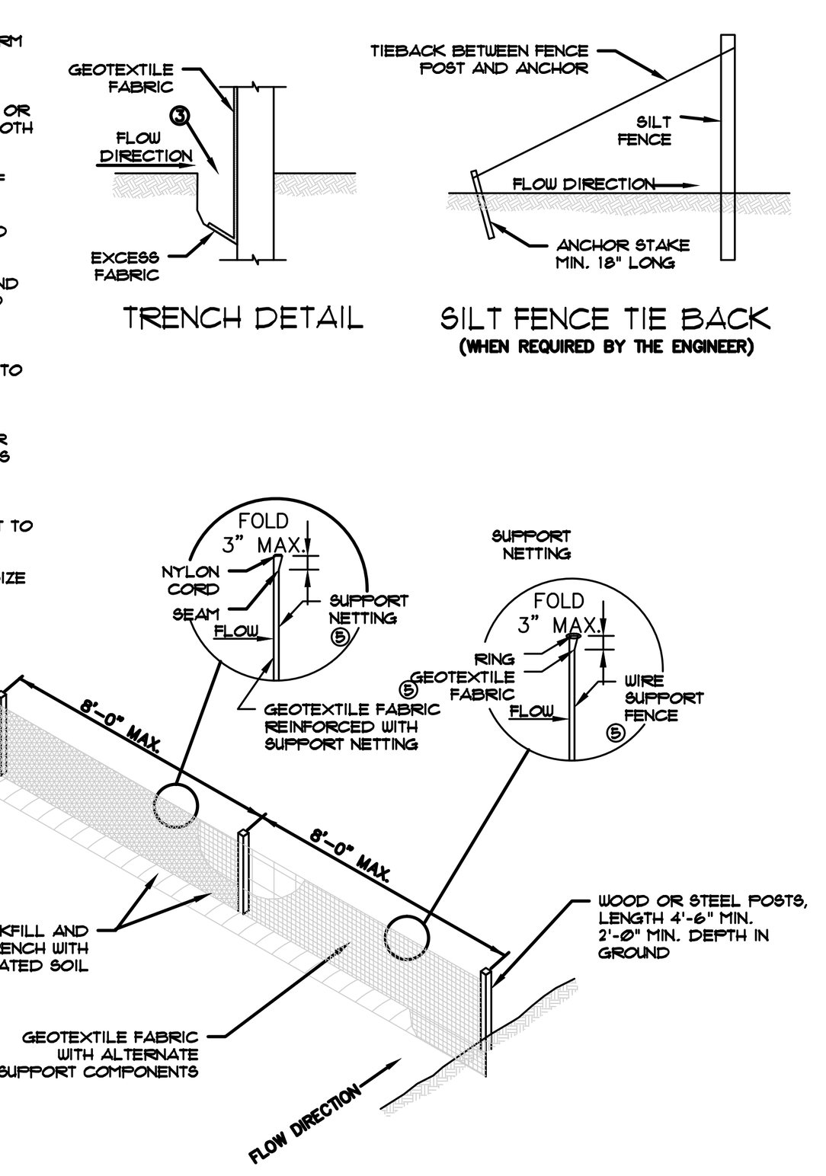
④ WIRE SUPPORT FENCE SHALL BE 14 GAGE MINIMUM WOVEN WIRE WITH A MAXIMUM MESH SPACING OF 6". SECURE TOP OF GEOTEXTILE FABRIC TO TOP OF FENCE WITH STAPLES OR WIRE RINGS AT 12" C-C.

⑤ GEOTEXTILE FABRIC SHALL BE REINFORCED WITH AN INDUSTRIAL POLY-PROPYLENE NETTING WITH A MAXIMUM MESH SPACING OF 3/4" OR EQUAL. A HEAVY DUTY NYLON TOP SUPPORT CORD OR EQUIVALENT IS REQUIRED.

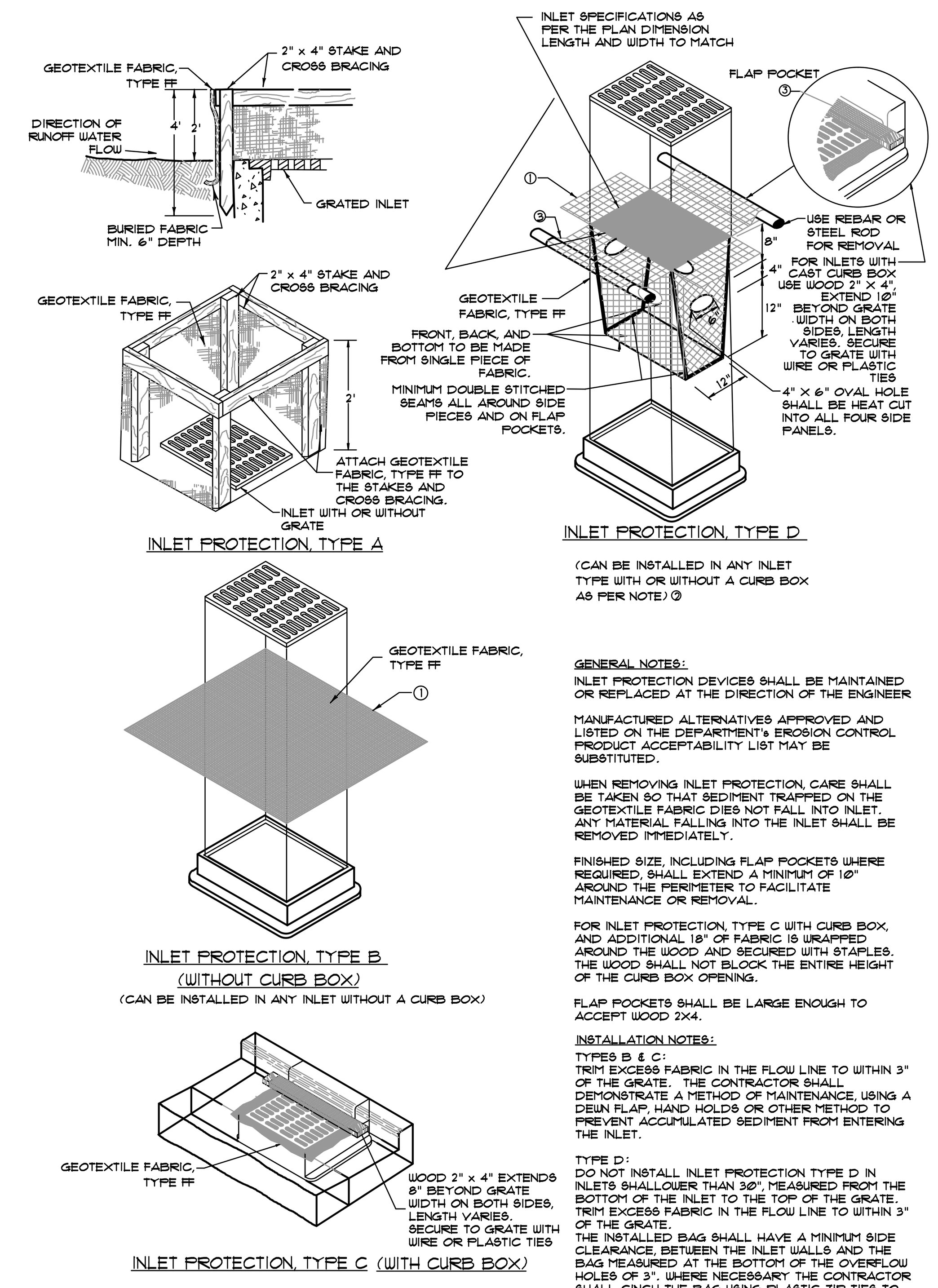
⑥ STEEL POSTS SHALL BE STUDDED "TEE" OR "U TYPE" WITH A MINIMUM WEIGHT OF 1.28 lbs/lin. ft. (WITHOUT ANCHOR). FIN ANCHORS SUFFICIENT TO RESIST POST MOVEMENT ARE REQUIRED. WOOD POSTS SHALL BE A MINIMUM SIZE OF 4" DIA. OR 1-1/2" x 3-1/2" EXCEPT WOOD POSTS FOR GEOTEXTILE FABRIC REINFORCED WITH NETTING SHALL BE A MINIMUM SIZE OF 1-1/8" x 1-1/8" OAK OR HICKORY.

ALTERNATIVES A AND B ARE EQUAL AND EITHER MAY BE USED.

NOTE: ADDITIONAL POST DEPTH OR TIE BACKS MAY BE REQUIRED IN UNSTABLE SOILS



01 SILT FENCE DETAIL 101
NTS 05/26/15



(CAN BE INSTALLED IN ANY INLET TYPE WITH OR WITHOUT A CURB BOX AS PER NOTE) ②

GENERAL NOTES:

INLET PROTECTION DEVICES SHALL BE MAINTAINED OR REPLACED AT THE DIRECTION OF THE ENGINEER

MANUFACTURED ALTERNATIVES APPROVED AND LISTED ON THE DEPARTMENT's EROSION CONTROL PRODUCT ACCEPTABILITY LIST MAY BE SUBSTITUTED.

WHEN REMOVING INLET PROTECTION, CARE SHALL BE TAKEN SO THAT SEDIMENT TRAPPED ON THE GEOTEXTILE FABRIC DIES NOT FALL INTO INLET. ANY MATERIAL FALLING INTO THE INLET SHALL BE REMOVED IMMEDIATELY.

FINISHED SIZE, INCLUDING FLAP POCKETS WHERE REQUIRED, SHALL EXTEND A MINIMUM OF 10" AROUND THE PERIMETER TO FACILITATE MAINTENANCE OR REMOVAL.

FOR INLET PROTECTION, TYPE C WITH CURB BOX, AND ADDITIONAL 18" OF FABRIC IS WRAPPED AROUND THE WOOD AND SECURED WITH STAPLES. THE WOOD SHALL NOT BLOCK THE ENTIRE HEIGHT OF THE CURB BOX OPENING.

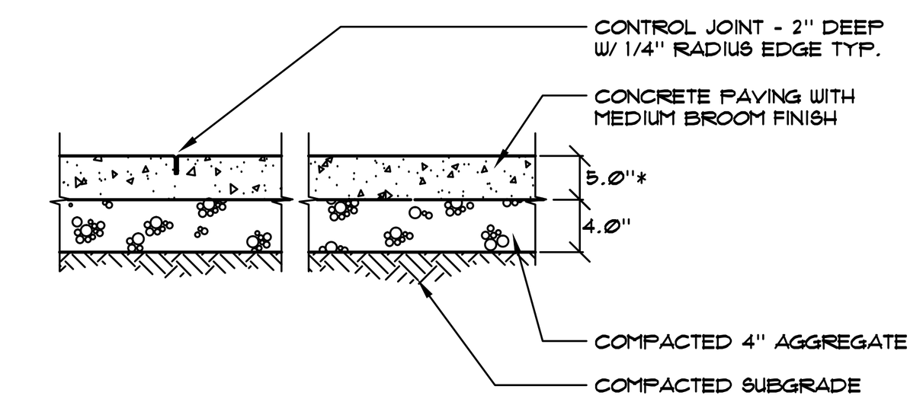
FLAP POCKETS SHALL BE LARGE ENOUGH TO ACCEPT WOOD 2X4.

INSTALLATION NOTES:

TYPES B & C:
TRIM EXCESS FABRIC IN THE FLOW LINE TO WITHIN 3" OF THE GRATE. THE CONTRACTOR SHALL DEMONSTRATE A METHOD OF MAINTENANCE, USING A DEWN FLAP, HAND HOLDS OR OTHER METHOD TO PREVENT ACCUMULATED SEDIMENT FROM ENTERING THE INLET.

TYPE D:
DO NOT INSTALL INLET PROTECTION TYPE D IN INLETS SHALLOWER THAN 30", MEASURED FROM THE BOTTOM OF THE INLET TO THE TOP OF THE GRATE. TRIM EXCESS FABRIC IN THE FLOW LINE TO WITHIN 3" OF THE GRATE.
THE INSTALLED BAG SHALL HAVE A MINIMUM SIDE CLEARANCE, BETWEEN THE INLET WALLS AND THE BAG MEASURED AT THE BOTTOM OF THE OVERFLOW HOLES OF 3". WHERE NECESSARY THE CONTRACTOR SHALL CINCH THE BAG USING PLASTIC ZIP TIES TO ACHIEVE THE 3" CLEARANCE. THE TIES SHALL BE PLACES AT A MAXIMUM IF 4" FROM THE BOTTOM OF THE BAG.

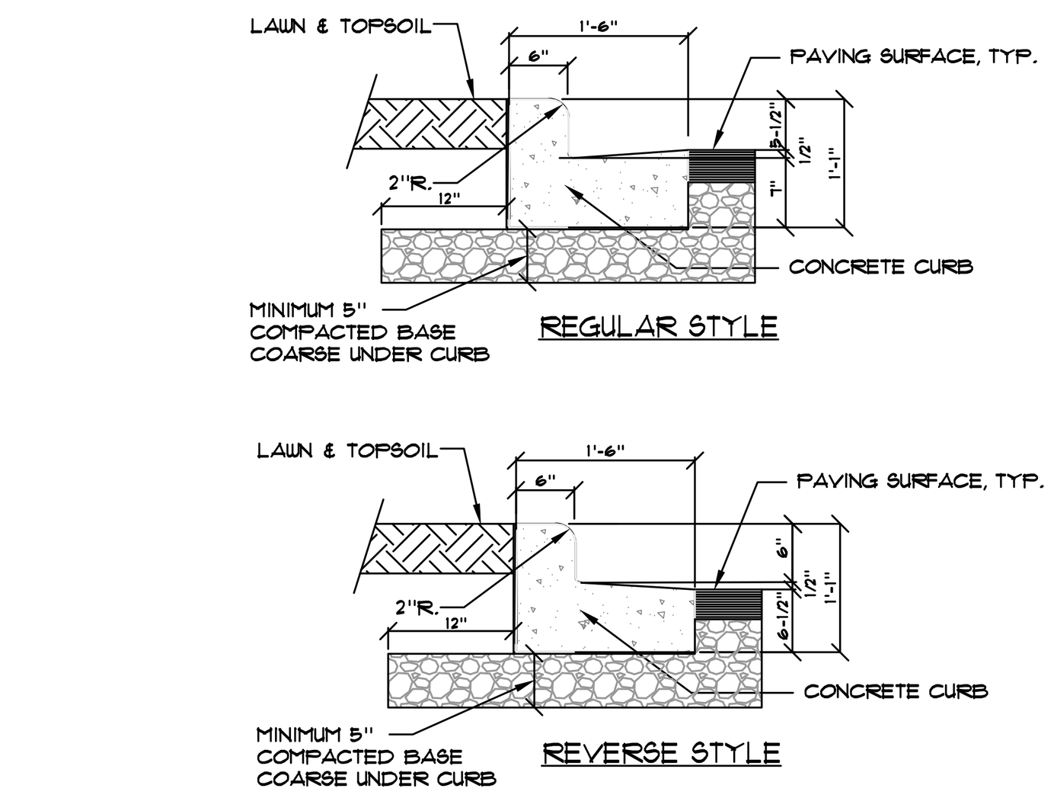
02 INLET PROTECTION 103
NTS 08/09/11



NOTES:
- CONTROL JOINT SPACING SHALL BE A MAXIMUM OF 5' AND CONSTRUCTED IN CONFORMANCE WITH AMERICAN CONCRETE INSTITUTE (ACI) RECOMMENDATIONS.
- EXPANSION JOINTS SHOULD BE PROVIDED WHERE PAVEMENT ABUTS FIXED OBJECTS.
- MATRIX 81 BLEND MICRO FIBER TO BE ADDED AT A RATE OF 1.5 POUNDS PER CUBIC YARD.

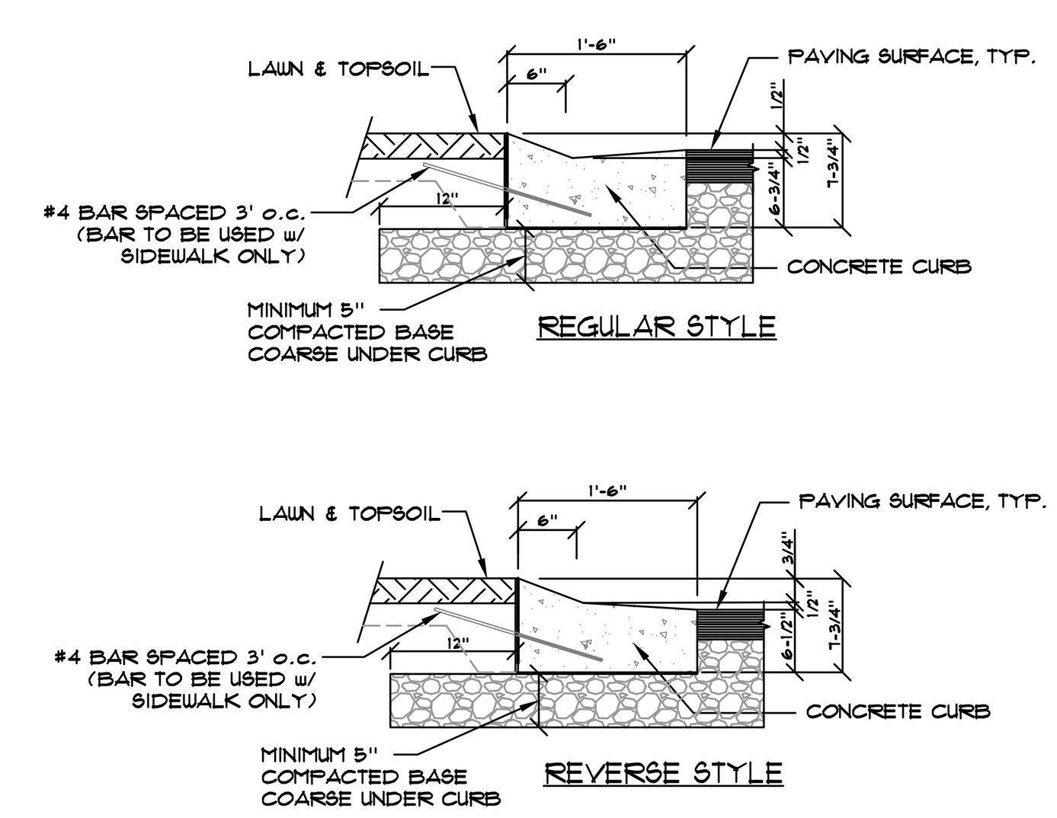
* NOTE:
SIDEWALK SHALL BE 7" AT DRIVEWAY SECTIONS

03 CONCRETE SIDEWALK SECTION 524
NTS 01/23/13



NOTE:
* REVERSE STYLE CURB LOCATIONS ARE NOTED ON THE PLANS.

04 VERTICAL FACE CURB - 18" WIDE 504
NTS 11/19/10



NOTE:
* REVERSE STYLE CURB LOCATIONS ARE NOTED ON THE PLANS.

05 18" DEPRESSED CURB 508
NTS 11/19/10

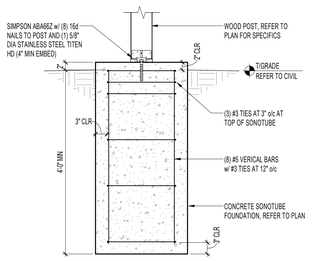
Harwood Engineering Consultants

MillerCoors
MILWAUKEE, WISCONSIN

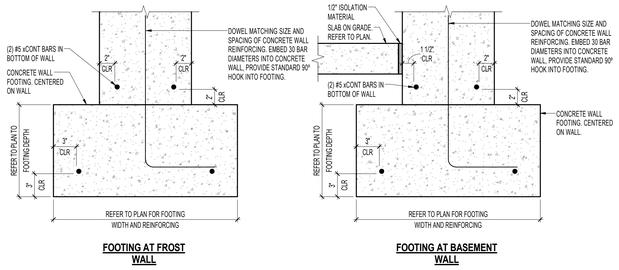
MILLERCOORS PROJECT NUMBER	122612	PROJECT DESIGN LEAD	ADLER		
PLANT:	MILWAUKEE	CONSTRUCTION DETAILS & SPECIFICATIONS			
DATE:	01/09/18	HISTORIC PRESERVATION SUBMITTAL			
JAK PROJECT NUMBER	17047-00	JAK PROJECT MANAGER	DK		

DESCRIPTION	REV.	CHANGED BY	DATE

	INITIAL	DATE	SUBJECT BLDG. NO. RELEASE NO. SIZE
DR.			C5.00
CH.			
APPR.			
SCALE			



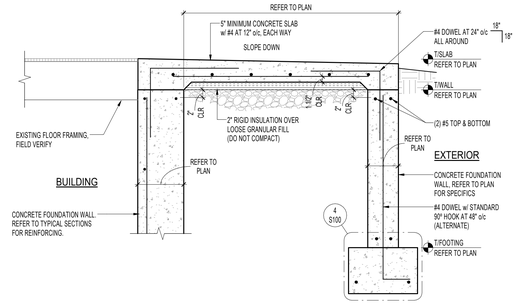
5 S100 SONOTUBE FOUNDATION



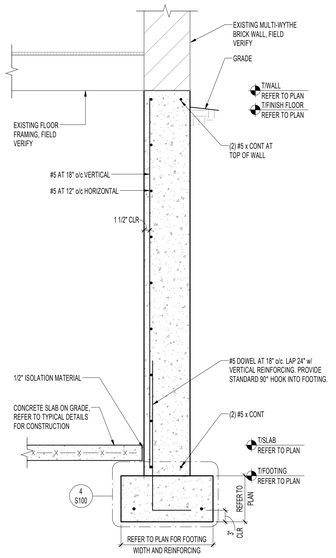
FOOTING AT FROST WALL

FOOTING AT BASEMENT WALL

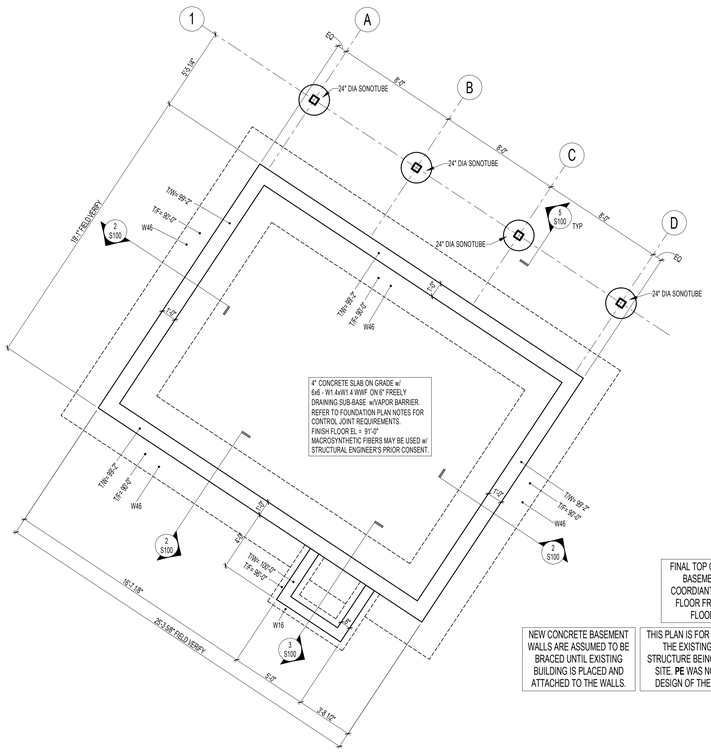
4 S100 TYPICAL WALL FOOTING DETAILS



3 S100 REINFORCED CONCRETE STOOP

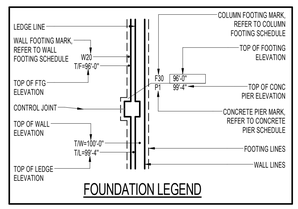


2 S100 PRECAST PLANK AT EXTERIOR CONCRETE FOUNDATION WALL



1 S100 FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

FOUNDATION PLAN NOTES:	FOUNDATION PLAN KEYED NOTES:
1. REFER TO GENERAL NOTES FOR ADDITIONAL STRUCTURAL NOTES AND FOUNDATION REQUIREMENTS. 2. ELEVATION 100'-0" ON STRUCTURAL DRAWINGS CORRESPONDS TO FF ELEVATION SHOWN ON SITE PLAN. TYPICAL. 3. SLAB ON GRADE CONTROL JOINTS: PROVIDE SAW CUT CONTROL JOINTS IN CONCRETE SLAB ON GRADE CONSTRUCTION WITHIN 24 HOURS OF INITIAL POUR. CONTROL JOINTS SHALL BE SPACED AT 36 TIMES THE SLAB THICKNESS, UP TO A MAXIMUM SPACING OF 14'-0". THE ASPECT RATIO OF SLAB PANELS SHALL BE A MAXIMUM OF 1.5 TO 1. CONTROL JOINTS SHALL BE PLACED ON COLUMN CENTERLINES, INTERIOR CORNERS, AND FLOOR DISCONTINUITIES (PITS, EQUIPMENT PADS, TRENCHES, DEPRESSED SLABS, ETC.). SLAB ON GRADE CONSTRUCTION SHALL CONFORM TO ACI 302 "GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION". REFER TO TYPICAL DETAILS FOR SLAB ON GRADE CONSTRUCTION. 4. BASED ON THE SOILS REPORT, CONTRACTOR TO BE AWARE OF AREAS OF POSSIBLE OVEREXCAVATION TO REMOVE POOR SOILS. SOIL BEARING CAPACITY IS TO BE FIELD VERIFIED BY GEOTECHNICAL ENGINEER PRIOR TO POURING ANY FOUNDATIONS.	1



FOUNDATION LEGEND

WALL FOOTING SCHEDULE					
	DIMENSIONS		REINFORCEMENT		
MARK	WIDTH (xCONT)	THICKNESS	LONGITUDINAL	TRANSVERSE	REMARKS
W16	1'-6"	1'-0"	(2) #5		
W46	4'-6"	1'-0"	(4) #5	#5 AT 16" o/c	

WALL FOOTING SCHEDULE NOTES:
1. REFER TO STRUCTURAL NOTES SHEET FOR MINIMUM COVER REQUIREMENTS.
2. REFER TO FOUNDATION PLAN FOR TOP OF FOOTING ELEVATIONS.
3. CONTRACTOR TO HIRE SOILS ENGINEER TO FIELD VERIFY AT TIME OF FOOTING EXCAVATION.
4. ALL LAPS IN STEEL REINFORCING SHALL BE CLASS "B" LAP SPLICES UNLESS NOTED OTHERWISE.

FINAL TOP OF NEW CONCRETE BASEMENT WALL TO BE COORDIANTED WITH EXISTING FLOOR FRAMING AND FINAL FLOOR ELEVATION.

NEW CONCRETE BASEMENT WALLS ARE ASSUMED TO BE BRACED UNTIL EXISTING BUILDING IS PLACED AND ATTACHED TO THE WALLS.

THIS PLAN IS FOR THE FOUNDATION FOR THE EXISTING 1856 GETTELMAN STRUCTURE BEING RELOCATED TO THIS SITE. **PE** WAS NOT INVOLVED IN THE DESIGN OF THE SUPERSTRUCTURE.

PIERCE ENGINEERS, INC.
CONSULTING STRUCTURAL ENGINEERS
161 N. Broadway Ave | Milwaukee, WI 53202
Phone: 414.278.6060 | Fax: 414.278.6061
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PE PROJECT: 17698

MillerCoors

MILLERCOORS PROJECT NUMBER	122612
PROJECT LEAD DESIGNER	ADLER
PLANT:	MILWAUKEE
DATE:	01/09/18
JAK PROJECT NUMBER	17047-00
JAK PROJECT MANAGER	DK

FOUNDATION PLAN
HISTORICAL PRESERVATION SUBMITTAL

DESCRIPTION	REV	BY	DATE	INITIAL	DATE
				DR.	
				CH.	
				APPR.	
				SCALE	

SUBJECT BLDG. NO. RELEASE NO. SIZE

S100

GENERAL NOTES:

1. ALL MATERIALS, CONSTRUCTION, AND DETAILS SHALL CONFORM WITH THE FOLLOWING:
   PLANS AND SPECIFICATIONS
   CODE AS SPECIFIED IN DESIGN DATA
   OSHA REGULATIONS
2. THE GENERAL CONTRACTOR AND SUBCONTRACTORS SHALL BE FAMILIAR WITH THE ENTIRE SET OF CONSTRUCTION DOCUMENTS (ARCHITECTURAL, CIVIL, ELECTRICAL, PLUMBING, STRUCTURAL, ETC.) IN ORDER TO PROVIDE ALL CONSTRUCTION AND MATERIALS FOR THIS PROJECT.
3. THE CONTRACTOR SHALL REFER TO OTHER DRAWINGS CONTAINED IN THE CONSTRUCTION DOCUMENTS FOR ADDITIONAL SPECIFIED MEMBERS, DIMENSIONS, ELEVATIONS, DETAILS, OPENINGS, INSERTS, SLEEVES, DEPRESSIONS, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS REQUIRED TO CONSTRUCT THIS PROJECT.
4. DETAILS SHOWN ON STRUCTURAL DRAWINGS SHALL BE APPLICABLE TO ALL PORTIONS OF THE CONTRACT DOCUMENTS UNLESS NOTED OTHERWISE.
5. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS.
6. DO NOT SCALE PLANS.
7. IN NO CASE SHALL STRUCTURAL ALTERATIONS OR WORK AFFECTING A STRUCTURAL MEMBER BE MADE UNLESS APPROVED BY THE STRUCTURAL ENGINEER.
8. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND CONSTRUCTION SEQUENCE IN ORDER TO ENSURE THE SAFETY OF THE BUILDING AND WORKMEN DURING CONSTRUCTION (MEANS & METHODS OF CONSTRUCTION). THIS INCLUDES, BUT IS NOT LIMITED TO: SHORING, UNDERPINNING, TEMPORARY BRACING, ETC.
9. CONSTRUCTION DOCUMENTS SHOW DIMENSIONS AND ELEVATIONS TO SIGNIFICANT WORKING POINTS (COLUMN CENTERLINES, OUTSIDE FACE OF WALLS, TOP OF FRAMING MEMBERS, ETC.) MATERIAL SUPPLIERS AND DESIGNERS ARE RESPONSIBLE FOR ALL OTHER INFORMATION IN ORDER TO DETAIL/FABRICATE THEIR WORK. CONTACT THE ARCHITECT WITH ANY DISCREPANCIES.
10. IN THE EVENT OF ANY DISCREPANCIES BETWEEN THE STRUCTURAL DRAWINGS AND ANY OTHER PLANS CONTAINED IN THIS SET OF CONSTRUCTION DOCUMENTS, THE CONTRACTOR SHALL BRING THE DISCREPANCY TO THE ARCHITECTS ATTENTION IN WRITING IMMEDIATELY.
11. NO PROVISIONS HAVE BEEN MADE IN THE DESIGN OF THIS STRUCTURE FOR FUTURE EXPANSION. UNLESS NOTED ON PLAN

EXISTING CONSTRUCTION/CONDITIONS:

1. ALL EXISTING FRAMING SHOWN ON THESE DRAWINGS IS BASED ON AVAILABLE DOCUMENTATION & FIELD OBSERVATION TO DATE. CONTRACTOR SHALL FIELD VERIFY ALL SIZES, DIMENSIONS, ELEVATIONS, AND CONFIGURATIONS OF EXISTING STRUCTURAL ELEMENTS (COLUMNS, BEAMS, WALLS, ETC.) AS NECESSARY TO PROPERLY INSTALL ALL NEW STRUCTURAL ELEMENTS AS SHOWN. COORDINATE DIFFERENCES BETWEEN FIELD CONDITIONS AND STRUCTURAL DRAWINGS WITH STRUCTURAL ENGINEER PRIOR TO PROCEEDING WITH WORK, AND PROCUREMENT/FABRICATION OF MATERIALS.
2. CONTRACTOR SHALL FIELD VERIFY EXISTING CONDITIONS AND NOTIFY ARCHITECT AND STRUCTURAL ENGINEER OF ANY CONFLICTS WITH CONSTRUCTION DOCUMENTS.
3. REMOVE AND REPLACE AND/OR MODIFY ALL EXISTING CONSTRUCTION (ARCHITECTURAL, STRUCTURAL, ELECTRICAL, AND MECHANICAL) AS REQUIRED IN ORDER TO PLACE NEW STRUCTURAL WORK SHOWN ON THE CONSTRUCTION DOCUMENTS. DO NOT MODIFY STRUCTURAL COMPONENTS UNLESS DETAILED ON THE CONSTRUCTION DOCUMENTS.
4. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND CONSTRUCTION SEQUENCE IN ORDER TO ENSURE THE SAFETY OF THE BUILDING AND WORKMEN DURING CONSTRUCTION (MEANS & METHODS OF CONSTRUCTION). THIS INCLUDES, BUT IS NOT LIMITED TO: SHORING, UNDERPINNING, TEMPORARY BRACING, ETC. CONTRACTOR SHALL DESIGN AND PROVIDE ALL SHORING REQUIRED TO SUPPORT EXISTING CONSTRUCTION AND NEW CONSTRUCTION AS REQUIRED TO BUILD THIS PROJECT.

FOUNDATION AND EARTHWORK:

1. ALL EXTERIOR FOOTINGS MUST BEAR BELOW LOCAL FROST LINE RELATIVE TO ADJACENT FINISH EXTERIOR GRADE.
2. DO NOT PLACE ANY FOOTINGS ON FROZEN SUBGRADE.
3. BACK FILLING SHALL BE DONE SIMULTANEOUSLY ON BOTH SIDES OF FOUNDATION WALLS.
4. DO NOT PLACE BACK FILL AGAINST BASEMENT WALLS UNTIL THE TOP AND BOTTOM OF THE WALL ARE ADEQUATELY BRACED BY THE SLAB ON GRADE AND THE FLOOR FRAMING AT THE TOP OF THE WALL.
5. REMOVE ANY EXISTING CONCRETE 2'-0" BELOW NEW CONCRETE FOOTINGS AND SLABS ON GRADE, UNLESS NOTED OTHERWISE.
6. SHORING/OR UNDERPINNING SHALL BE DESIGNED TO LIMIT HORIZONTAL AND VERTICAL MOVEMENT OF EXISTING CONSTRUCTION TO 1/4" MAXIMUM IN ANY DIRECTION.
7. CENTER PIER AND COLUMN FOOTINGS ON COLUMN CENTERLINES AND WALL FOOTINGS ON WALL CENTERLINES UNLESS SPECIFICALLY NOTED OTHERWISE.
8. ALL BACK FILL WITHIN 3'-0" OF RETAINING WALLS AND BASEMENT WALLS SHALL BE FREE DRAINING GRANULAR MATERIAL APPROVED BY A SOILS ENGINEER AND COMPACTED TO 90% STANDARD PROCTOR.
9. TOP OF FOOTING ELEVATIONS SHOWN ON THESE CONSTRUCTION DOCUMENTS REPRESENT MINIMUM FOOTING DEPTHS FOR FROST PROTECTION AND BEST JUDGMENT OF A SUITABLE BEARING STRATUM. ACTUAL GRADE CONDITIONS AND SUITABLE BEARING STRATUM MUST BE VERIFIED BY THE CONTRACTOR AND A SOILS ENGINEER AT THE TIME OF EXCAVATION.
10. FOOTING EXCAVATIONS MUST EXTEND TO COMPETENT BEARING MATERIAL. CONTRACTOR SHALL HIRE A SOILS ENGINEER TO FIELD VERIFY NET ALLOWABLE SOIL BEARING CAPACITY STATED ON THESE CONSTRUCTION DOCUMENTS AND IN GEOTECHNICAL REPORT FOR THIS PROJECT. IF SUITABLE BEARING STRATUM DOES NOT EXIST AT FOOTING ELEVATIONS STATED ON CONSTRUCTION DOCUMENTS, EXCAVATIONS SHALL BE EXTENDED UNTIL SOIL WITH STATED BEARING CAPACITY IS REACHED. PLACE COMPACTED FILL BELOW FOOTINGS OR EXTEND FOOTINGS DOWN TO SUITABLE BEARING STRATUM. ENGINEERED FILL BELOW SLABS ON GRADE AND FOOTINGS SHALL BE FREE DRAINING GRANULAR MATERIAL COMPACTED TO 95% MODIFIED PROCTOR AND PLACED PER THE SOIL ENGINEERS RECOMMENDATIONS. ALL FIELD CONDITIONS THAT WILL AFFECT DESIGN AS PRESENTED MUST BE COORDINATED WITH STRUCTURAL ENGINEER.
11. REFER TO DESIGN DATA FOR DESCRIPTION OF SOIL CONDITIONS, GEOTECHNICAL RECOMMENDATIONS, AND DESIGN VALUES.
12. WHERE NEW FOOTINGS ABUT EXISTING FOOTINGS, STEP OR THICKEN THE NEW FOOTING AS REQUIRED TO HAVE NEW BOTT/FTG ELEVATION MATCH EXISTING BOTT/FTG ELEVATION. CONTRACTOR SHALL FIELD VERIFY EXISTING BOTT/FTG ELEVATION.

CAST-IN-PLACE REINFORCED CONCRETE:

1. CONCRETE WORK SHALL CONFORM TO REFERENCED EDITION OF ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" AND ACI 302 "GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION".
2. CONTRACTOR SHALL ELECTRONICALLY SUBMIT STEEL REBAR SHOP DRAWINGS FOR APPROVAL PRIOR TO CONSTRUCTION. CONTRACTOR SHALL REVIEW AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTING TO THE ARCHITECT.
3. GROUT BELOW BASE PLATES AND BEARING PLATES SHALL BE NON-SHRINK, NON-METALLIC GROUT
4. STEEL REINFORCING BARS SHALL CONFORM TO ASTM A615 (GRADE 60). PLAIN WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064.
5. CONTRACTOR SHALL PROVIDE SUITABLE WIRE SPACERS, CHAIRS, TIES, ETC FOR SUPPORTING REINFORCING STEEL IN THE PROPER POSITION WHILE PLACING CONCRETE.
6. PROVIDE (2)-#5 BARS AROUND ALL OPENINGS AND (2)-#5 BARS DIAGONALLY AT ALL OPENING CORNERS. EXTEND BARS PER DETAILS.
7. PROVIDE 1/2" EXPANSION JOINT MATERIAL AT INTERIOR LOCATIONS WHERE SLABS ABUT WALLS, COLUMNS, AND OTHER VERTICAL SURFACES UNLESS NOTED OTHERWISE.
8. PROVIDE A 1" CHAMFER ON EXPOSED CORNERS OF CONCRETE UNLESS NOTED OTHERWISE. TOP SURFACE OF WALLS SHALL FINISHED SMOOTH, UNLESS NOTED OTHERWISE.
9. DO NOT PLACE CONDUITS, PIPES, DUCTS, OR FIXTURES IN STRUCTURAL CONCRETE UNLESS NOTED OTHERWISE.
10. SLEEVES, CONDUITS, OR PIPING PASSING THROUGH CONCRETE SLABS AND WALLS SHALL BE PLACED SO THAT THEY ARE NOT CLOSER THAN THREE DIAMETERS ON CENTER OR 4" MIN AND SO THAT THEY DO NOT DISPLACE REINFORCING. BANKS OF OPENINGS GREATER THAN 18" TOTAL WIDTH OF ALL OPENINGS EDGE-TO-EDGE MUS BE COORDINATED WITH STRUCTURAL ENGINEER.
11. CONTROL JOINTS SHALL BE PLACED IN SLAB ON GRADE AND SLAB ON METAL DECK CONSTRUCTION WITHIN 24 HOURS OF INITIAL POUR. REFER TO PLAN NOTES FOR ADDITIONAL INFO.
12. CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIR OF ANY IRREGULARITIES OR DEFECTS IN CONCRETE SLABS (CRACKS, BUMPS, FLOOR CURLING, ETC.) BEFORE ANY FLOOR FINISHES ARE APPLIED.
13. REFER TO REINFORCEMENT DEVELOPMENT AND LAP SPLICE SCHEDULE FOR LAP SPLICES IN REINFORCING STEEL.
14. STEEL REINFORCING SPLICES OF ADJACENT BARS SHALL BE STAGGERED SUCH THAT SPLICES ARE 4 FEET APART, MINIMUM.
15. ALL LAPS IN REINFORCING STEEL SHALL BE CLASS "B" LAP SPLICES UNLESS OTHERWISE NOTED.
16. CONTRACTOR SHALL HIRE A MATERIALS TESTING LABORATORY TO CAST AND TEST CONCRETE CYLINDERS. ALL TESTING SHALL BE IN ACCORDANCE WITH ACI 318. RESULTS OF CYLINDER TESTS SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER. CONCRETE TEST REPORTS SHALL STATE THE FOLLOWING INFORMATION:

    LOCATION ON PROJECT WHERE THE CONCRETE IS USED
    7 DAY COMPRESSIVE STRENGTH
    28 DAY COMPRESSIVE STRENGTH
    AIR CONTENT
    SLUMP
    AMOUNT OF WATER ADDED ON JOB SITE
    MIX USED
17. CONCRETE TEST REPORTS SHALL DIRECTLY STATE WHETHER OR NOT THE TEST RESULT COMPLIES WITH THE CONSTRUCTION DOCUMENTS AND SPECIFICATIONS.
18. ADDITION OF JOBSITE WATER TO CONCRETE SHALL BE PER ASTM C94.
19. TIME BETWEEN CONCRETE BATCHING AND PLACEMENT SHALL BE IN ACCORDANCE WITH ASTM C94.
20. CLASS C FLY ASH OR SLAG MAY BE SUBSTITUTED FOR CEMENT ON A POUND TO POUND BASIS. SUBMITTED MIX DESIGNS SHALL INDICATE SUBSTITUTION ARE AND IS SUBJECT TO ENGINEER APPROVAL.
21. ALL CONCRETE SLABS SHALL BE CURED PER ACI RECOMMENDATIONS FOR NO LESS THAN SEVEN DAYS OR AN APPROPRIATE CURING COMPOUND MAY BE APPLIED.
22. CALCIUM CHLORIDE OR ADMIXTURES CONTAINING CALCIUM CHLORIDE ARE NOT PERMITTED IN ANY CONCRETE MIX.
23. PROVIDE THE FOLLOWING CLEAR COVER DISTANCES FOR REINFORCEMENT IN CONCRETE:

    CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"

    CONCRETE EXPOSED TO EARTH OR WEATHER:
    NO. 6 THROUGH NO. 18 BARS 2"
    NO. 5 BAR AND SMALLER 1 1/2

    CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
    SLABS, WALLS, JOISTS: NO. 11 BAR AND SMALLER 1"
    BEAMS AND COLUMNS 1 1/2"
24. CONTRACTOR SHALL USE SMOOTH FORMS FOR EXPOSED CONCRETE SURFACES. ANY CONCRETE SURFACE REPAIRS SHALL BE PERFORMED BY THE CONTRACTOR AS REQUIRED. REPAIR AND PATCH DEFECTIVE AREAS WITH PROPRIETARY PATCHING COMPOUND IMMEDIATELY AFTER REMOVAL OF FORMS.

STRUCTURAL STEEL:

1. DESIGN, FABRICATION, AND ERECTION SHALL CONFORM TO AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION) "STEEL CONSTRUCTION MANUAL". EDITIONS AS SPECIFIED BY CODE.
2. STEEL DETAILING AND CONNECTIONS SHALL CONFORM TO THE REQUIREMENTS OF AISC 360 "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS". EDITIONS AS SPECIFIED BY CODE.
3. WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS HOLDING CURRENT AWS CERTIFICATES IN THE TYPES OF WELDING SPECIFIED ON THESE CONSTRUCTION DOCUMENTS.
4. CONTRACTOR SHALL ELECTRONICALLY SUBMIT STEEL SHOP DRAWINGS FOR APPROVAL PRIOR TO FABRICATION. CONTRACTOR SHALL REVIEW AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTING TO THE ARCHITECT.
5. CONTRACTOR SHALL DETERMINE, FURNISH AND INSTALL ANY TEMPORARY BRACING OR GUYS REQUIRED TO ERECT STEEL MEMBERS. TEMPORARY BRACING SHALL BE LEFT IN PLACE UNTIL THE PERMANENT STRUCTURE IS IN PLACE AND SECURE. REFER TO PLAN NOTES FOR DESCRIPTION OF LATERAL SYSTEM.
6. PROVIDE 3/16" CAP PLATE AT THE ENDS OF ALL EXPOSED TUBE AND PIPE MEMBERS, UNLESS NOTED OTHERWISE.
7. STAIRS, HANDRAILS, AND GUARDRAILS SHALL BE DESIGNED BY THE STEEL SUPPLIER. CONNECTIONS INTO SURROUNDING STRUCTURE SHALL BE APPROVED BY STRUCTURAL ENGINEER. CALCULATIONS OF ALL STAIR COMPONENTS MUST BE SUPPLIED WITH STAIR SHOP DRAWINGS.
8. ALL STEEL BEAMS SHALL BE FABRICATED WITH THE NATURAL CAMBER (WITHIN MILL TOLERANCE) IN THE UPWARD VERTICAL DIRECTION.
9. THE STEEL SUPPLIER SHALL COORDINATE HIS WORK WITH THE STEEL JOIST SUPPLIER ON THE PROJECT.
10. STEEL CONNECTIONS SHALL BE DETAILED AS INDICATED ON THE CONSTRUCTION DOCUMENTS. WHERE CONNECTIONS ARE NOT DETAILED. THE FABRICATOR IS ALLOWED TO DETAIL THE CONNECTION BASED ON THE BEAM REACTIONS (WHERE SHOWN) SHOWN AND THE FOLLOWING
    A. ALL REACTIONS ARE SERVICE-LOAD LEVEL
    B. ASD SELECTION OF THE CONNECTION MATERIALS
    C. USE OF STANDARDS SHOWN ON CONSTRUCTION DRAWINGS DETAILS AS A GUIDE FOR CONNECTION SELECTION.

    ALTERNATE CONNECTIONS FROM WHAT IS SPECIFIED ON THE CONSTRUCTION DOCUMENTS WILL NOT BE ACCEPTED WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER.
11. PROVIDE STIFFENER PLATES ON BOTH SIDES OF BEAM WEBS AT ALL CONCENTRATED LOADS ABOVE AND BELOW A BEAM. UNLESS NOTED OTHERWISE, FRAME THE LARGEST BEAM OVER COLUMNS AT BEAM TO BEAM INTERSECTIONS.
12. ANY HOLES, CUTS, OR COPING FIELD CUT INTO STEEL MUST BE VERIFIED w/ STRUCTURAL ENGINEER PRIOR TO WORK. CONTRACTOR SHALL COORDINATE ALL HOLES REQUIRED BY OTHERS w/ STRUCTURAL ENGINEER.

WOOD FRAMING:

1. DESIGN, FABRICATION, AND CONSTRUCTION SHALL CONFORM TO THE CURRENT EDITION OF "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", AMERICAN FOREST AND PAPER ASSOCIATION.
2. DESIGN, FABRICATION, AND CONSTRUCTION OF ALL PLYWOOD FRAMING SHALL CONFORM TO THE CURRENT EDITION OF "PLYWOOD DESIGN SPECIFICATIONS", AMERICAN PLYWOOD ASSOCIATION.
3. PLYWOOD SHEATHING SHALL CONFORM TO THE CURRENT EDITION OF "U.S. PRODUCT STANDARD PS-1" FOR SOFTWOOD PLYWOOD AND BEAR THE APA GRADE-TRADEMARK OF THE AMERICAN PLYWOOD ASSOCIATION.
4. PLYWOOD SHEATHING SHALL BE ATTACHED TO WOOD FRAMING WITH THE LONG DIMENSION OF THE SHEATHING LAID PERPENDICULAR TO THE SUPPORTS. STAGGER ALL JOINTS.
5. PLYWOOD SHEATHING SHALL BE FASTENED TO SUPPORTS w/ 10d NAILS SPACED AT 6" o/c AT PANEL EDGES AND 12" o/c AT INTERMEDIATE SUPPORTS UNLESS NOTED OTHERWISE.
6. ANY PLYWOOD SHEATHING THAT IS EXPOSED TO MOISTURE SHALL BE PRESSURE TREATED.
7. PLYWOOD PANEL EDGES SHALL BEAR ON THE FRAMING SUPPORT MEMBERS AND BUTT ALONG THEIR CENTER LINES. NAILS SHALL BE PLACED NOT LESS THAN 3/8" IN FROM THE PANEL EDGE.
8. WOOD SILL PLATES AND OTHER WOOD MEMBERS DIRECTLY EXPOSED TO MOISTURE OR IN DIRECT CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED.
9. MAXIMUM MOISTURE CONTENT IN ANY WOOD MEMBER SHALL NOT EXCEED 19%.
10. 2x WOOD JOISTS SHALL HAVE 1x3 SPF NO.2 CROSS BRIDGING AT 8'-0" o/c MAXIMUM.
11. DO NOT EMBED WOOD MEMBERS IN CONCRETE.
12. ALL BOLTS AND LAG SCREWS SHALL CONFORM TO ASTM A307 UNLESS NOTED OTHERWISE. USE STEEL WASHERS BETWEEN HEAD OF BOLT OR LAG SCREW AND WOOD. USE STEEL WASHERS BETWEEN NUT AND WOOD.
13. ALL FASTENERS ATTACHING PRESSURE TREATED WOOD MEMBERS TO CONCRETE OR MASONRY SHALL BE HOT DIPPED GALVANIZED OR STAINLESS STEEL.
14. MAKE NO SUBSTITUTIONS OF ANY PRODUCTS SPECIFIED ON ANY FRAMING PLANS WITHOUT THE DIRECT WRITTEN PERMISSION OF THE STRUCTURAL ENGINEER AND ARCHITECT.
15. TEMPORARY BRACING SHALL BE PROVIDED AND REMAIN IN PLACE UNTIL THE STRUCTURE IS COMPLETELY STABLIIZED. TO RESIST BUCKLING OF LOAD BEARING STUDS, USE A CONTINUOUS 2x FRAMING MEMBER ATTACHED TO THE STUD WALL AT MID-HEIGHT. USE TEMPORARY X-BRACING TO RESIST LATERAL WIND AND SEISMIC LOADS. PROVIDE ANY OTHER TEMPORARY BRACING DEEMED NECESSARY DURING CONSTRUCTION. BRACING MAY BE REMOVED ONCE THE SHEATHING IS APPLIED TO AT LEAST ONE SIDE OF THE STUDS. TEMPORARY BRACING IS THE RESPONSIBILITY OF THE WOOD FRAMER.
16. ARCHITECT AND CONTRACTOR SHALL DETAIL AND CONSTRUCT BUILDING FINISHES TO ACCOMMODATE AN EXPECTED BUILDING SHRINKAGE OF APPROXIMATELY 3/16" TO 3/8" PER FLOOR OF WOOD CONSTRUCTION. PROPER CARE SHALL BE TAKEN TO PREVENT STORED AND INSTALLED LUMBER FROM THE ELEMENTS. DO NOT ALLOW LUMBER TO REST IN STANDING WATER.

METAL PLATE CONNECTED WOOD TRUSSES:

1. WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE CURRENT EDITIONS OF "DESIGN SPECIFICATIONS FOR METAL PLATE CONNECTED WOOD TRUSSES" BY TRUSS PLATE INSTITUTE (TPI) AND "NATIONAL DESIGN SPECIFICATIONS FOR STRESS-GRADE LUMBER AND ITS FASTENINGS" BY NATIONAL FOREST PRODUCTS ASSOCIATION.
2. ROOF TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS:
    TOP CHORD LIVE LOAD — REFER TO ROOF SNOW LOAD PLAN
    TOP CHORD DEAD LOAD — 10 PSF
    BOTTOM CHORD LIVE LOAD — 10 PSF (NON CONCURRENT w/ TOP CHORD LIVE LOAD)
    BOTTOM CHORD DEAD LOAD — 10 PSF
3. FLOOR TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS:
    TOP CHORD LIVE LOAD — REFER TO BUILDING DESIGN LIVE LOADS
    TOP CHORD DEAD LOAD — 10 PSF
    BOTTOM CHORD LIVE LOAD — 10 PSF (NON CONCURRENT w/ TOP CHORD LIVE LOAD)
    BOTTOM CHORD DEAD LOAD — 10 PSF
4. IN ADDITION TO THE LOADS STATED ABOVE THE TRUSSES SHALL BE DESIGNED FOR ANY SNOW DRIFTING, MECHANICAL, AND/OR ANY SPECIAL LOAD CONDITIONS AS SHOWN ON STRUCTURAL PLANS AND AS REQUIRED BY THE BUILDING CODE AS SPECIFIED IN "DESIGN DATA" SECTION.
5. ROOF TRUSSES SHALL HAVE A MAXIMUM LIVE LOAD DEFLECTION OF L/360.
6. FLOOR TRUSSES SHALL HAVE A MAXIMUM LIVE LOAD DEFLECTION OF L/480.
7. FABRICATION, HANDLING, STORAGE, AND ERECTION SHALL BE IN ACCORDANCE WITH "TRUSS PLATE INSTITUTION" RECOMMENDED PRACTICES AND SHALL BE DONE IN A WORKMAN LIKE MANNER SO AS TO NOT DAMAGE THE TRUSSES. TRUSSES SHALL NOT BE CUT, ADDED ONTO OR ALTERED IN ANY WAY WITH OUT THE WRITTEN CONSENT OF THE TRUSS DESIGNER, ENGINEER, AND ARCHITECT.
8. WOOD TRUSS DESIGNER/SUPPLIER SHALL SUBMIT FORMAL STAMPED CALCULATIONS BY A REGISTERED ENGINEER IN THE STATE OF WISCONSIN FOR REVIEW BEFORE FABRICATION.
9. CONTRACTOR SHALL ELECTRONICALLY SUBMIT TRUSS SHOP DRAWINGS FOR APPROVAL PRIOR TO FABRICATION. CONTRACTOR SHALL REVIEW AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTING TO THE ARCHITECT.
10. SHOP DRAWING SUBMISSIONS SHALL INCLUDE THE FOLLOWING INFORMATION:
    THE NAME, ADDRESS, PHONE NUMBER, AND FAX NUMBER OF THE SUPPLIER.
    SLOPE OR DEPTH, SPAN AND SPACING
    LOCATION OF ALL JOINTS
    ALL DESIGN LOADS
    ADJUSTMENTS TO LUMBER AND METAL CONNECTOR PLATE VALUES FOR CONDITIONS OF USE
    EACH REACTION FORCE AND DIRECTION
    METAL CONNECTOR PLATE TYPE, SIZE, GAUGE, AND THE DIMENSIONAL LOCATION OF EACH CONNECTOR PLATE
    LUMBER SIZE, SPECIES, AND GRADE FOR EACH TRUSS MEMBER
    CONNECTION REQUIREMENTS FOR TRUSS TO TRUSS GIRDER, TRUSS PLY TO PLY, AND FIELD SPLICES.
    CALCULATED DEFLECTION RATIO AND/OR MAXIMUM DEFLECTION FOR LIVE AND TOTAL LOAD
    SPECIFY ALL TRUSS TO TRUSS CONNECTIONS AND HANGERS.
    SPECIFY AND SHOW ALL PERMANENT TRUSS BRACING REQUIRED BY DESIGN.
11. CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION PROCEDURES AND TEMPORARY TRUSS BRACING REQUIREMENTS DURING ERECTION IN ACCORDANCE WITH TPI's COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING, AND BRACING METAL PLATE CONNECTED WOOD TRUSSES (HIB-91 BOOKLET) AND THE CURRENT EDITION OF ANSI/TPI-1.
12. TRUSSES EXPOSED TO MOISTURE SHALL BE CONSTRUCTED OF PRESSURE TREATED WOOD AND GALVANIZED METAL PLATES.
13. FLOOR TRUSS SPACING SHOWN ON FRAMING PLANS ARE MAXIMUM SPACINGS. TRUSS DESIGNER SHALL REDUCE SPACING AS REQUIRED TO SUPPORT ALL LOADS SPECIFIED ON THESE PLANS AND BY CODE.
14. DESIGN ROOF TRUSSES TO RESIST ALL WIND LOADS INCLUDING UPLIFT AS REQUIRED BY THE INTERNATIONAL BUILDING CODE-2009 WITH SEPTEMBER 1, 2011 WISCONSIN AMENDED I-CODE INSERTS. MINIMUM NET UPLIFT = 10 PSF, 20 PSF AT CANOPIES & OVERHANGS.
15. ALL TRUSS TO TRUSS CONNECTIONS ARE TO BE DESIGNED, DETAILED, AND SUPPLIED BY THE TRUSS SUPPLIER.
16. TRUSS FABRICATOR SHALL FIELD VERIFY ALL SPAN DIMENSIONS BEFORE FABRICATING.

DESIGN DATA

APPLICABLE CODES/STANDARDS:
.....INTERNATIONAL BUILDING CODE - 2009 WITH SEPTEMBER 1, 2011 WISCONSIN AMENDED I-CODE INSERTS
.....INTERNATIONAL EXISTING BUILDING CODE - 2009
.....ASCE 7-05 MIN DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, ASCE/SEI

STRUCTURAL DESIGN STANDARDS (DESIGN SHALL CONFORM TO THE CURRENT EDITION UNDER THE APPLICABLE CODE)
.....ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND COMMENTARY
.....ACI 530/530.1 BUILDING CODE REQUIREMENTS AND SPECS FOR MASONRY STRUCTURES (AND RELATED COMMENTARIES)
.....ANSI/AISC 360 SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS
.....AWS D1.1/D1.1M STRUCTURAL WELDING CODE-STEEL
.....NDS-NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION ASD/LRFD
.....NDS-NATIONAL DESIGN SPECIFICATION SUPPLEMENT, DESIGN VALUES FOR WOOD CONSTRUCTION
.....AISI S100 NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS
.....AISI S213 NORTH AMERICAN SPECIFICATION FOR COLD-FORMED STEEL FRAMING-LATERAL DESIGN

BUILDING DESIGN LOADS/CRITERIA

DESIGN DEAD LOADS:	
.....FIRST FLOOR DEAD LOAD (ASSUMED)	20 psf
.....UPPER FLOOR DEAD LOAD (ASSUMED)	20 psf
.....ROOF DEAD LOAD (ASSUMED)	20 psf
DESIGN LIVE LOADS:	
.....FLOOR FRAMING (RETAIL, OFFICE, RESTAURANT, RECREATIONAL)	100 psf
.....STAIRWAYS, CORRIDORS, LOBBIES (OTHER AREAS)	100 psf
.....DECKS	100 psf

HANDRAIL ASSEMBLIES & GUARDS
.....200LB LOAD OR 50 PLF LOAD APPLIED IN ANY DIRECTION AT TOP OF HANDRAIL ASSEMBLY OR GUARD
.....& TO TRANSFER THIS LOAD THROUGH SUPPORTS TO THE STRUCTURE

ROOF SNOW LOADS & DESIGN DATA:	
.....DESIGN ROOF SNOW LOAD	25 psf (BALANCED SNOW LOAD)
.....FLAT ROOF SNOW LOAD (Pf) = (0.7*Ce*Ct*Is*Pg)	24.5 psf
.....SNOW EXPOSURE FACTOR (Ce)	1.0
.....SNOW LOAD IMPORTANCE FACTOR (Is)	1.0
.....ROOF THERMAL FACTOR (Ct)	1.0
.....GROUND SNOW (Pg)	35 psf
.....RAIN ON SNOW SURCHARGE	0
.....SLOPED ROOF FACTOR (Cs)	1.0
WIND DESIGN DATA:	
.....WIND IMPORTANCE FACTOR (Iw)	1.0
.....BASIC WIND SPEED (3-SECOND GUST)	90 MPH
.....WIND DIRECTIONALITY FACTOR (Kd)	0.85
.....MEAN ROOF HEIGHT	21 FT
.....WIND EXPOSURE CATEGORY	B
.....WIND EXPOSURE CLASSIFICATION	ENCLOSED
.....INTERNAL PRESSURE COEFFICIENT	+/-0.18
.....BUILDING LENGTH (L)	25.25 FT
.....LEAST WIDTH (B)	19 FT
.....VELOCITY PRESSURE EXPOSURE COEFFICIENT Kh (CASE 1)	0.701
.....VELOCITY PRESSURE EXPOSURE COEFFICIENT Kh (CASE 2)	0.636
.....TOPOGRAPHIC FACTOR (Kzt)	1.0
.....EDGE STRIP (a)	3.0 FT
.....END ZONE (2a)	6.0 FT
.....DESIGN PROCEDURE	METHOD 1 (SIMPLIFIED PROCEDURE)

WIND LOADS COMPONENTS & CLADDING

ROOF SURFACE PRESSURE

AREA	10 SF	50 SF	100 SF
NEGATIVE ZONE 1	-13.3 psf	-12.5 psf	-12.1 psf
NEGATIVE ZONE 2	-23.2 psf	-18.9 psf	-17.0 psf
NEGATIVE ZONE 3	-34.3 psf	-29.1 psf	-26.9 psf
POSITIVE ALL ZONES	10.0 psf	10.0 psf	10.0 psf
OVERHANG ZONE 1&2	-27.2 psf	-27.2 psf	-27.2 psf
OVERHANG ZONE 3	-45.7 psf	-35.3 psf	-30.9 psf

WALL SURFACE PRESSURE

AREA	10 SF	100 SF	500 SF
NEGATIVE ZONE 4	-15.8 psf	-13.6 psf	-12.1 psf
NEGATIVE ZONE 5	-19.5 psf	-15.1 psf	-12.1 psf
POSITIVE ZONE 4&5	14.6 psf	12.4 psf	10.9 psf

EARTHQUAKE DESIGN DATA:	
.....OCCUPANCY CATEGORY	II
.....SEISMIC IMPORTANCE FACTOR (Ie)	1
.....MAPPED SPECTRAL ACCELERATIONS AT SHORT PERIODS (Ss)	0.107
.....MAPPED SPECTRAL ACCELERATIONS AT (1) SECOND PERIODS (S1)	0.044
.....SITE CLASSIFICATIONS	D
.....DESIGN SPECTRAL RESPONSE COEFFICIENT AT SHORT PERIODS (Sds)	0.114
.....DESIGN SPECTRAL RESPONSE COEFFICIENT AT (1) SECOND PERIODS (Sd1)	.070
.....SEISMIC DESIGN CATEGORY	B
.....BASIC SEISMIC-FORCE-RESISTING SYSTEM	STRUCTURE NOT SPECIFICALLY DETAILED FOR SEISMIC RESTANCE
.....DESIGN BASE SHEAR	0.038W KIPS
.....SEISMIC RESPONSE COEFFICIENT (Cs)	0.038
.....RESPONCE MODIFICATION COEFFICIENT	3
.....ANALYSIS PROCEDURE FOR SEISMIC DESIGN	EQUIVALENT LATERAL FORCE ANALYSIS
.....BUILDING IS IN MILWAUKEE COUNTY	
SOIL DESIGN VALUES:	
.....SOIL UNIT WEIGHT	110 PCF (ASSUMED)
.....LATERAL EARTH PRESSURE	
..........ACTIVE (RETAINING WALLS)	40 PSF/FT OF DEPTH (ASSUMED)
..........AT-REST (BASEMENT WALLS)	60 PSF/FT OF DEPTH (ASSUMED)
..........PASSIVE	300 PSF (ASSUMED)
.....COEFFICIENT OF SLIDING FRICTION	0.30 (ASSUMED)
.....SUBGRADE MODULUS	150 PCI (ASSUMED)
.....ALLOWABLE SOIL BEARING PRESSURE	1,500 PSF (ASSUMED)

REFER TO SOILS REPORT NO. XXXX DATED XXXX PREPARED BY XXXX FOR DESCRIPTION OF SOIL CONDITIONS, GEOTECHNICAL RECOMMENDATIONS, AND DESIGN VALUES

DEFLECTION LIMITS

MEMBERS	LIVE	SNOW or WIND	DEAD + LIVE or SNOW
ROOF MEMBERS			
SUPPORTING GYPSUM BOARD CEILINGS	L/360	L/360	L/240
SUPPORTING FLEXIBLE CEILINGS	L/360	L/360	L/240
NOT SUPPORTING CEILING	L/240	L/240	L/180
SUPPORTING RIGID MATERIALS (BRICK, MASONRY, ETC.)	L/600	L/600	L/600
FLOOR MEMBERS			
SUPPORTING RIGID MATERIALS (BRICK, MASONRY, ETC.)	L/600	N/A	L/600
SUPPORTING FLEXIBLE MATERIALS	L/360	N/A	L/240
LINTEL/HEADER/BEAM MEMBERS			
SUPPORTING RIGID MATERIALS (BRICK, MASONRY, ETC.)	L/600	L/600	L/600
SUPPORTING FLEXIBLE MATERIALS	L/360	L/360	L/240
EXTERIOR WALLS			
WITH RIGID FINISHES (BRICK, MASONRY, ETC.)	N/A	L/600	N/A
WITH FLEXIBLE FINISHES (EIFS, SIDING, ETC.)	N/A	L/360	N/A

MATERIAL STRENGTHS

CAST-IN-PLACE CONCRETE:

FOOTINGS	
.....MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 3,000 PSI
.....MAXIMUM WATER-CEMENTITIOUS RATIO	0.59
.....MAXIMUM AGGREGATE SIZE	1 1/2"
.....SLUMP LIMIT	5" +/-1"
.....AIR CONTENT	NO
FOUNDATION FROST WALLS	
.....MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
.....MAXIMUM WATER-CEMENTITIOUS RATIO	0.48
.....MAXIMUM AGGREGATE SIZE	3/4"
.....SLUMP LIMIT	4" +/-1"
.....AIR CONTENT	YES 4% to 6%
EXTERIOR PIERS, WALLS, AND COLUMNS	
.....MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
.....MAXIMUM WATER-CEMENTITIOUS RATIO	0.48
.....MAXIMUM AGGREGATE SIZE	3/4"
.....SLUMP LIMIT	4" +/-1"
.....AIR CONTENT	YES 4% to 6%
INTERIOR SLABS ON GRADE	
.....MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
.....MAXIMUM WATER-CEMENTITIOUS RATIO	0.48
.....MAXIMUM AGGREGATE SIZE	3/4"
.....SLUMP LIMIT	4" +/-1"
.....AIR CONTENT	NO
EXTERIOR SLABS ON GRADE	
.....MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
.....MAXIMUM WATER-CEMENTITIOUS RATIO	0.48
.....MAXIMUM AGGREGATE SIZE	3/4"
.....SLUMP LIMIT	4" +/-1"
.....AIR CONTENT	YES 4% to 6%
SONOTUBES	
.....MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
.....MAXIMUM WATER-CEMENTITIOUS RATIO	0.50
.....MAXIMUM AGGREGATE SIZE	3/4"
.....SLUMP LIMIT	4" +/-1"
.....AIR CONTENT	NO
SLURRY	
.....MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 1,000 PSI
.....MAXIMUM WATER-CEMENTITIOUS RATIO	0.55
.....MAXIMUM AGGREGATE SIZE	1 1/2"
.....SLUMP LIMIT	6" +/-1"
.....AIR CONTENT	NO

FIBER REINFORCEMENT:
MACROSYNTHETIC FIBERS ENGINEERED & DESIGNED FOR USE IN CONCRETE SLABS COMPLYING WITH ASTM C 1116, TYPE III, 1 1/2" TO 2 1/2" LONG

STEEL/METAL:

REINFORCING STEEL:	
.....ALL ASTM A615, GRADE 60, DEFORMED	Fy = 60,000 PSI
.....STEEL WELDED WIRE REINFORCEMENT, FLAT SHEETS	Fy = 60,000 PSI
STRUCTURAL STEEL:	
.....ROLLED WIDE FLANGE SHAPES, ASTM A992 GRADE 50	Fy = 50,000 PSI
.....CHANNELS, ANGLES, AND S SHAPES, ASTM A36	Fy = 36,000 PSI
.....PLATE AND BAR, ASTM A36	Fy = 36,000 PSI
.....TUBE SHAPES, ASTM A500 GRADE B	Fy = 46,000 PSI
.....PIPE ASTM A53, TYPE E or S, GRADE B	Fy = 46,000 PSI
.....ALL OTHER ROLLED SHAPES, ASTM A36	Fy = 36,000 PSI
STRUCTURAL BOLTS:	
.....HIGH STRENGTH BOLTS, NUTS, & WASHERS	ASTM A325
.....ZINC-COATED HIGH STRENGTH BOLTS, NUTS, & WASHERS	ASTM A325
.....STAINLESS STEEL BOLTS, NUTS, & WASHERS	ASTM F593
.....SHEAR CONNECTORS (GRADES 1015 THRU 1020)	ASTM A108
.....THREADED RODS	ASTM A36
.....CLEVIS & TURNBUCKLES (GRADE 1035)	ASTM A108
.....EYE BOLTS & NUTS (GRADE 1030)	ASTM A108
.....ANCHOR BOLTS (GRADE 36)	ASTM F1554
WELDED CONNECTIONS:	
.....WELDING ELECTRODES	E70XX E80XX FOR WELDING REINF

WOOD FRAMING (UNO ON PLANS/DETAILS)

DIMENSIONAL LUMBER:	
.....JOISTS/BEAMS/HEADERS	SPRUCE-PINE-FIR No. 2 or BETTER
.....EXTERIOR LUMBER	TREATED SOUTHERN PINE No 2 or BETTER
.....POSTS/COLUMNS	SPRUCE-PINE-FIR No. 2 or BETTER
LAMINATED VENEER LUMBER (LVL):	
.....JOISTS/BEAMS/HEADERS	
.........E = 2,000 ksi	Fc (PARALLEL) = 2,510 psi
.........Fb = 2,600 psi	Fc (PERPENDICULAR) = 750 psi
.........Fv = 285 psi	
PARALLEL STRAND LUMBER (PSL):	
.....JOISTS/BEAMS/HEADERS	
.... E = 2,000 ksi	Fc (PARALLEL) = 2,900 psi
.........Fb = 2,900 psi	Fc (PERPENDICULAR) = 625 psi
.........Fv = 290 psi	
LAMINATED STRAND LUMBER (LSL):	
.....JOISTS/BEAMS/HEADERS	
.........E = 1,550 ksi	Fc (PARALLEL) = 2,170 psi
.........Fb = 2,325 psi	Fc (PERPENDICULAR) = 900 psi
.........Fv = 310 psi	

PIERCE ENGINEERS, INC.
CONSULTING STRUCTURAL ENGINEERS
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Phone: 414.278.6060 | Fax: 414.278.6061
www.pierceengineers.com
PE PROJECT: 17698

DESCRIPTION	REV	BY	DATE

MillerCoors™
MILLERCOORS PROJECT NUMBER: 122612
PROJECT LEAD DESIGNER: ADLER
PLANT: MILWAUKEE — GENERAL NOTES
DATE: 01/09/18 — HISTORICAL PRESERVATION SUBMITTAL

JAK architecture + interior design
JAKnetter Architects
N16 W23217 STONE RIDGE DRIVE, SUITE 300 WAUKESHA, WI 53188
www.jaknetter.com | office 262 513 9800 | fax 262 513 9815

JAK PROJECT NUMBER: 17047-00
JAK PROJECT MANAGER: DK

	INITIAL	DATE
DR.		
CH.		
APPR.		
SCALE		

SUBJECT BLDG. NO. RELEASE NO. SIZE

S001